CONTENTS

The external dimensions of the container from the nearest basic type of container the documentation corresponds derived:

Pos.:	Sign Nr.:	C O N T E N T :
1	unlabeled	Container frame
2	unlabeled	Roof construction
3	unlabeled	Dachtraute
4	unlabeled	Fussbodenkonstruktion
5	unlabeled	Stiel
6	unlabeled	Obenstirnseite der Dachtraufe

MATERIALS USED:
Steel: correspond ST 37
Trapezoidal plates: ROOF:
(Galvanized) 900*30*0.55 mm ČSN 42 6880
Reinforcing steel in accordance BST 420 S (III S)

REGULATIONS:
DIN 1055 Part 1 Loads for Buildings
DIN 1055 Part 2 W.V., soil characteristics ...
DIN 1055 Part 3 W.V.; traffic loads
DIN 1055 Part 4 W.V.; wind loads
DIN 1055 Part 5 W.V.; snow and ice
DIN 18800 Part 6 steel structures; Design
DIN 18800 Part 7 WV, manufacture, quality assurance for welding
ENV 1993-1-1 Design of Steel Structures, CEN Brussels. 1992

STATIC CALCULATION OF CONTAINERS
The calculation includes the following static evidence of container structures, which are used as recreation rooms and unterkiegen so don listed rules.

The support structure for the container design was modeled for the purpose of static analysis according to the scheme as a spatial model of the object.

The calculation was carried out with the program for the analysis of spatial constructions FEAT4. With this program, the basic analysis of the resulting values is carried out.

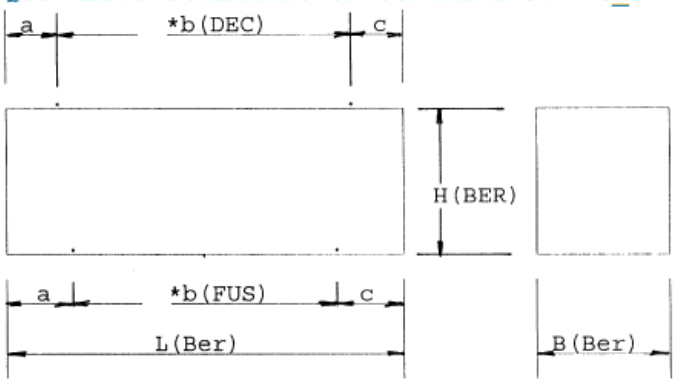
In the two-story columns or walls, the objects in the middle third of lower container is required.

In the two-story facilities with the metropolitan area on the ground floor the arrangements of the columns cross-sections and the upper Längsumfangsträger be carried out.

be performed for the multi-storey properties and for the objects with the ambitious plan, the new static calculation.

be carried out also for other strains, the new static analysis.

EQUIVALENT SCHEMATIC OF CONTAINERS: C_1



When the container length Change is vorausgezetzt that the dimensions remain "a" and "b" constant. In Aussenmasseveränderung more cross bars to be removed per the distances "b" were added (). Degree "c" will be recalculated more pieces.

ENTRY VALUES FOR THE CONTAINER (SPACE CELL):

C.Nummer.: 1× (6,058 × 2,438 × 2,820) m

DIMENSIONS:	Production:	Calculated:
L_{FER} :	6,058 m	L_{BER} : 5,946 m
B_{FER} :	2,438 m	B_{BER} : 2,326 m
H_{FER} :	2,820 m	H_{BER} : 2,674 m

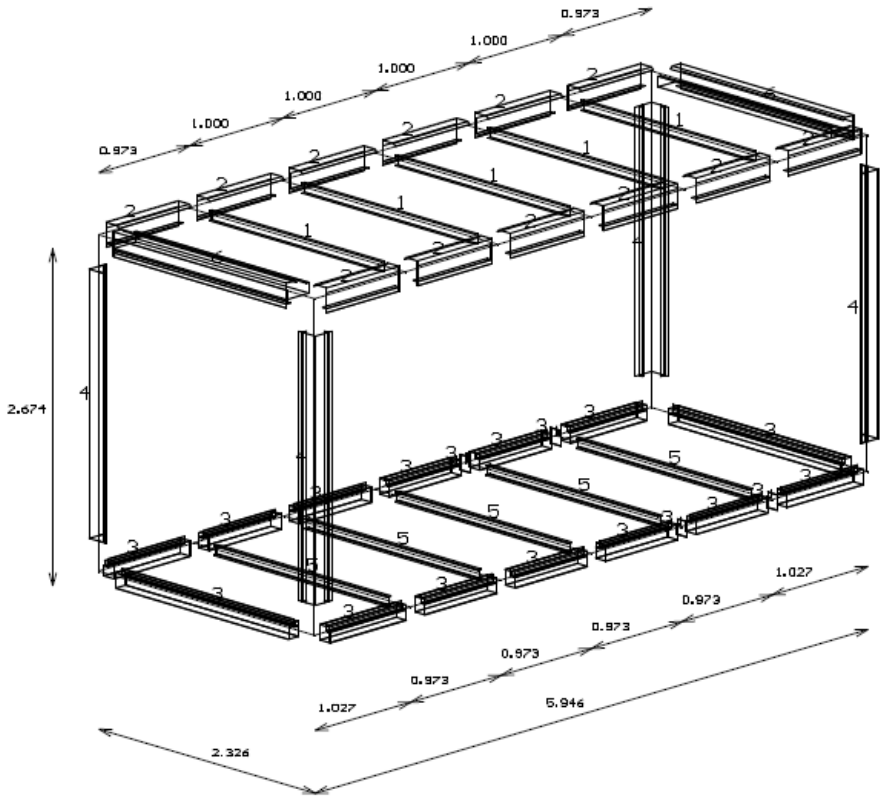
CG:

$e_{(L)}$:	0,056 m	$e_{(O)}$: 0,080 m
$e_{(B)}$:	0,056 m	$e_{(U)}$: 0,066 m

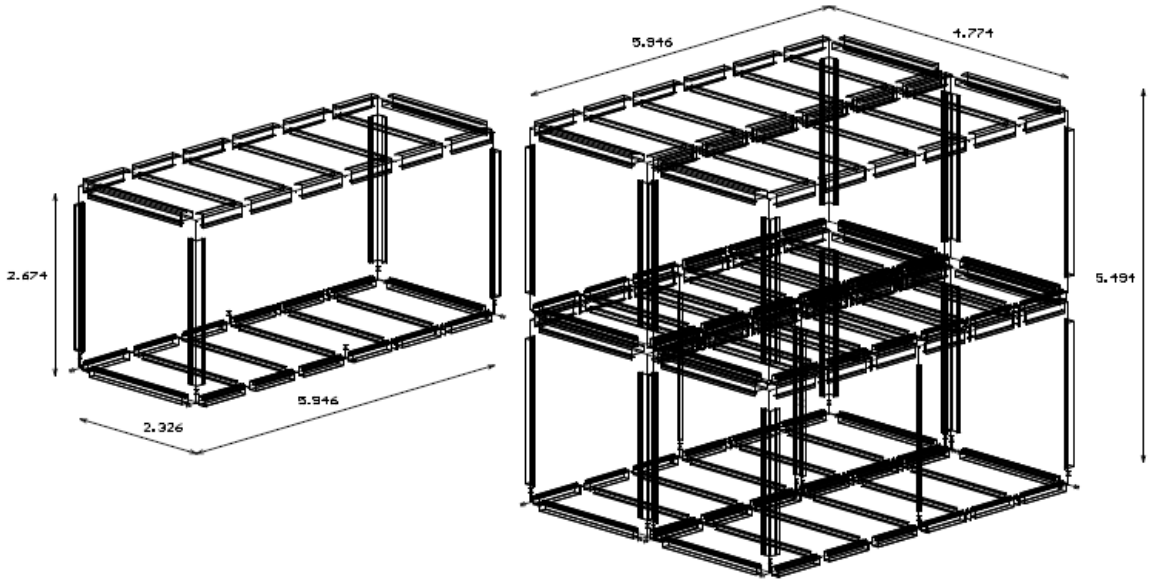
a_{KRAN} : 0,000 m
H_{KRAN} : 5,246 m

	n_{DECK}:	a_{DECK}:	b_{DECK}:	c_{DECK}:
Spacing of joists:	5	0,973 m	1,000 m	0,973 m
	n_{FUSS}:	a_{FUSS}:	b_{FUSS}:	c_{FUSS}:
Spacing of floor beams:	5	1,027 m	0,973 m	1,027 m

WORK SCHEME OF CONTAINERS
DIMENSION - POINT CROSS SECTION:



WORK SCHEME OF CONTAINERS:

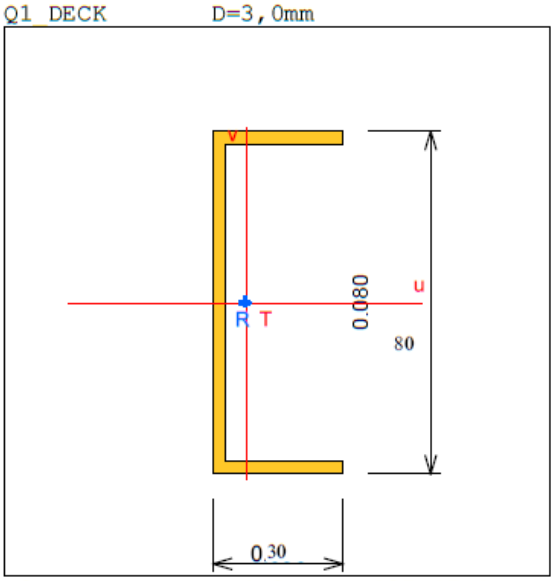


LISTING OF CASES LAST CALCULATED:

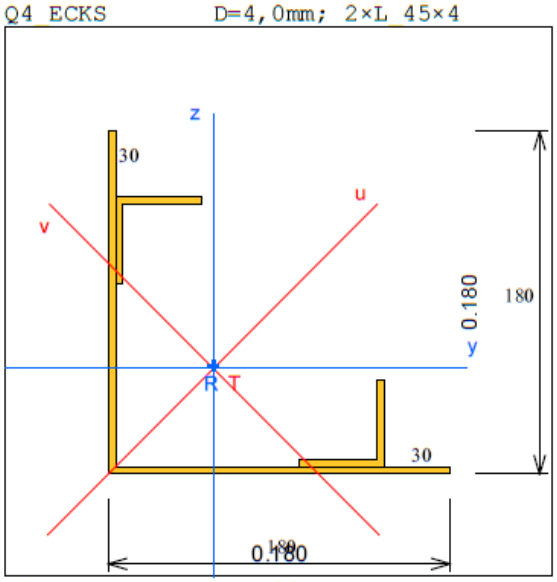
Iy, Iz	[m4]	principal moments of inertia
Ik	[m4]	torsion constant
beta y, beta z		Shear indulgence coefficients
P		voller Quersch.
S		zusamenges.
D		particularity

Section	Typ	Material	Area [m2]	Iy [m4]	Iz [m4]	Ik [m4]	IOmega	Beta y	Beta z
Q1_DECK	P	Fe360	4.020e-04	3.682e-07	3.178e-08	1.632e-09	0.400	0.575	
Q2_OLRA	S	Fe360	2.043e-03	8.653e-06	3.073e-06	3.715e-08	0.841	0.661	
Q3_UL&QRA	S	Fe360	1.562e-03	3.573e-06	2.672e-06	2.382e-08	0.469	0.535	
Q4_ECKS	S	Fe360	2.112e-03	7.495e-06	7.495e-06	3.318e-08	0.762	0.590	
Q5_FUSS	P	Fe360	7.640e-04	8.010e-07	8.490e-08	7.000e-09	0.626	0.398	
Q6_OQRA	S	Fe360	2.418e-03	7.332e-06	1.935e-05	3.202e-08	0.434	0.448	

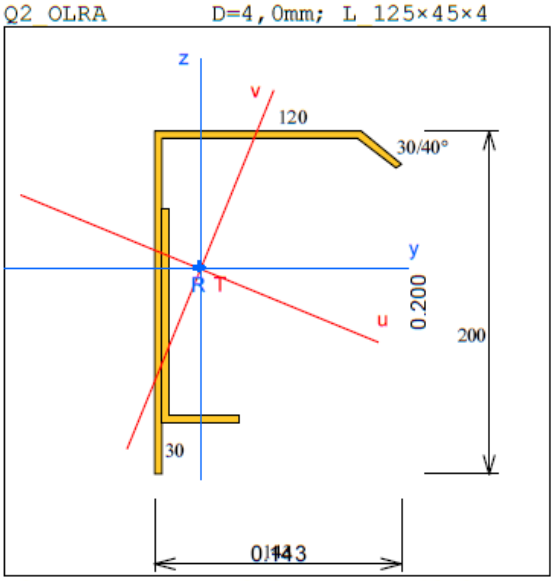
Q1 DECK D=3,0mm



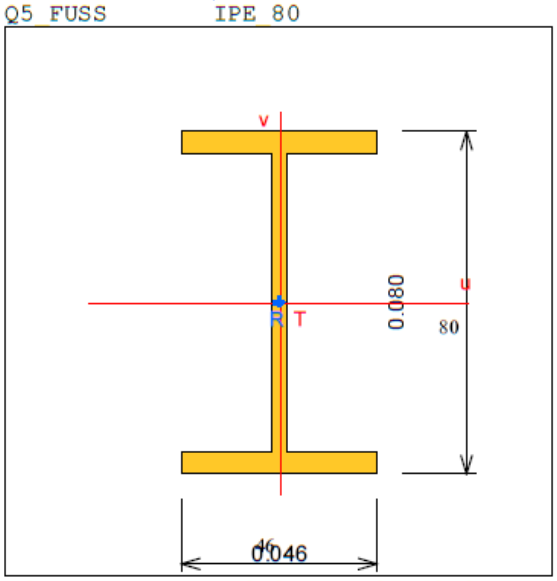
Q4 ECKS D=4,0mm; 2×L 45×4



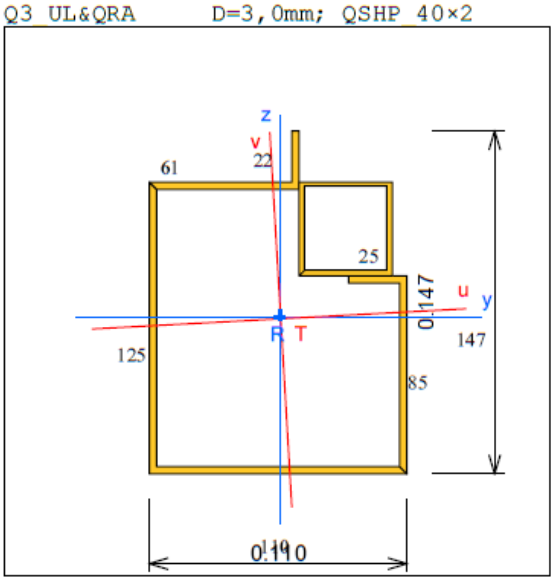
Q2 OLRA D=4,0mm; L 125×45×4



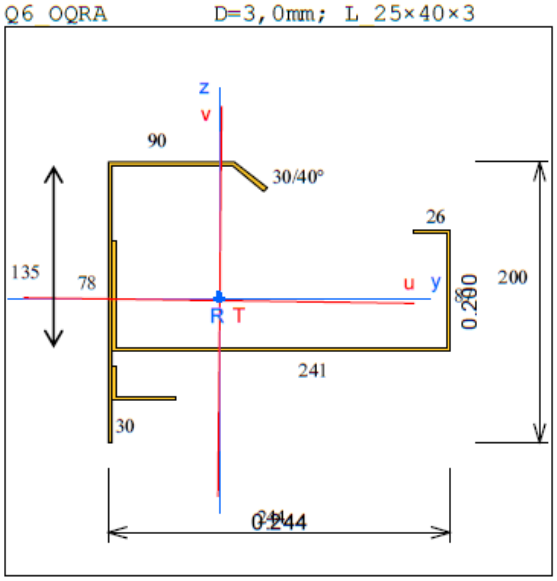
Q5 FUSS IPE 80



Q3 UL&QRA D=3,0mm; QSHP 40×2



Q6 OQRA D=3,0mm; L 25×40×3



LAST ADOPTION

L2 - ROOF:	m(i):	t(i):	n.f.:	g(n):
	kN/m^3	m		kN/m²
zinc-coated sheet metal:	78,5	0,00055	1,000	0,043
hardboard:	9,5	0,01300	1,000	0,124
thermal insulation:	1,0	0,08000	1,000	0,080
Steam-proof isolation:	22,0	0,00010	1,000	0,002
Frame:	0,0132	1,00000	2,500	0,033
hardboard:	9,0	0,01300	1,000	0,117
			Total load:	0,399
			Value calculation:	0,400

L2 - FLOOR DESIGN:	m(i):	t(i):	n.f.:	g(n):
	kN/m³ (kN/m²)	m		kN/m²
PVC Bodenbelag:	14,0	0,00200	1,000	0,028
hardboard:	9,5	0,02200	1,000	0,209
Steam-proof isolation:	22,5	0,00010	1,000	0,002
Frame:	0,0332	1,00000	2,500	0,083
thermal insulation:	1,0	0,08000	1,000	0,080
zinc-coated sheet metal:	78,5	0,00055	1,000	0,043
			Total load:	0,445
			Value calculation:	0,500

L2-GEWICHT DES AUSSENWANDMANTELS:	B(W) =	1,000 m		
	m(i):	t(i):	n.f.:	g(n):
	kN/m^3	m		kN/m²
zinc-coated sheet metal:	78,5	0,00055	1,100	0,047
Frame ; t= 60	78,5	0,00075	2,000	0,014
thermal insulation:	0,5	0,10000	1,000	0,050
Plaster board:	11,5	0,01250	1,000	0,144
			1,000	0,000
			1,000	0,000
			Total load:	0,255
Load of 1 m of wall height	3,069 m	g =	0,783	kN/m'
		Value calculation:	0,800	kN/m'

L3 - PAYLOAD ON THE FLOOR: $p_1 = 2,50$ kN/m²

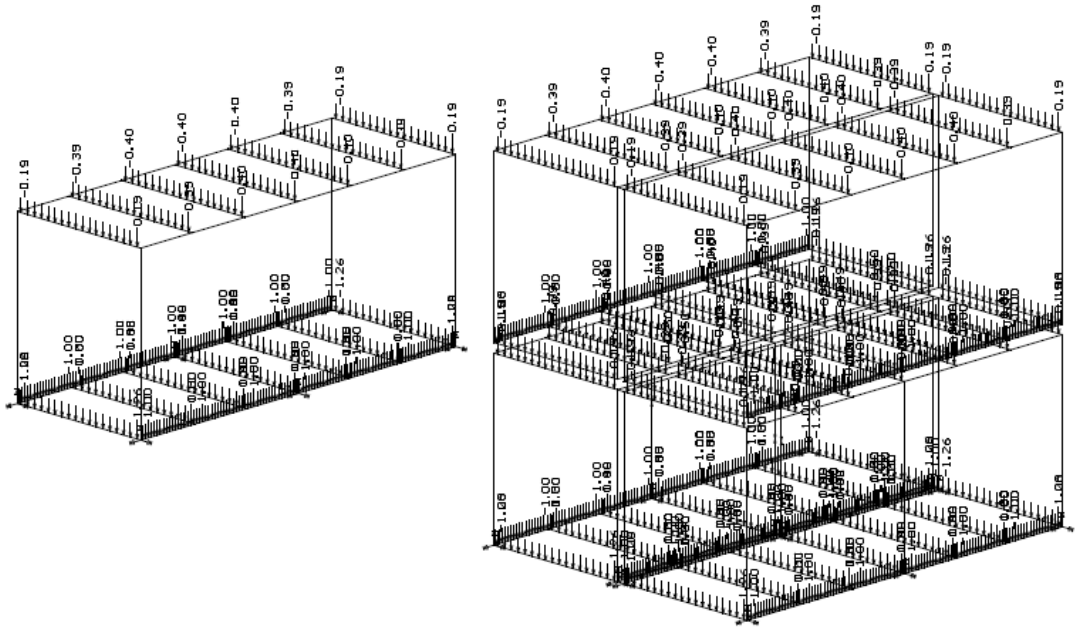
L4 - PAYLOAD ON THE ROOF: $s_0 = 1,25$ kN/m²

L5-9 - SNOW LOAD:

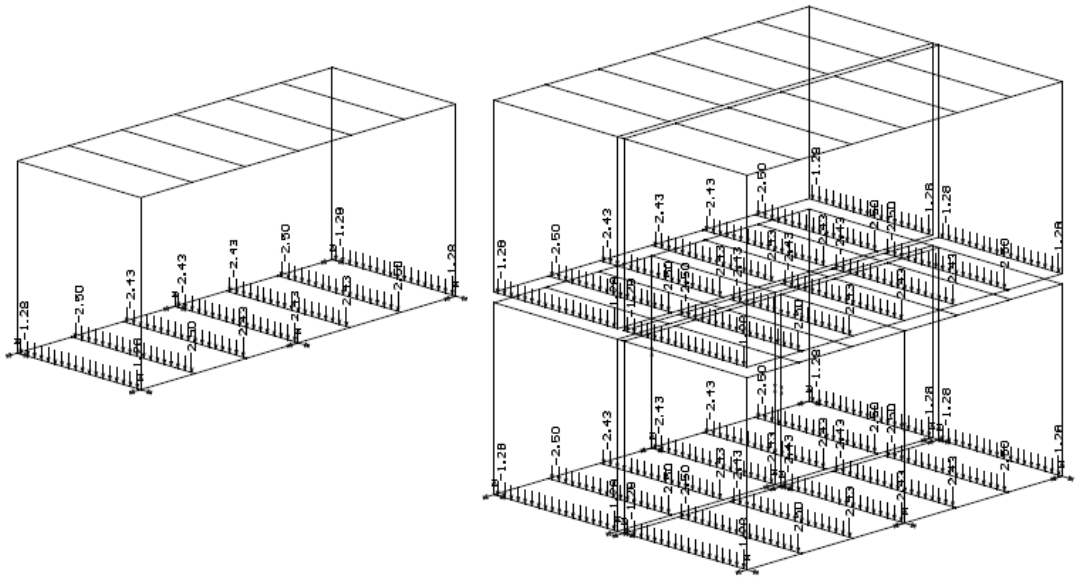
H < 8,0 m	w = 0,5	kN/m²				
$L_{Komplex}$=	13,960 m	$B_{KOMPLEX}$=	4,100 m	$H_{(Komplex)}$=	2,800 m	1 Stock
h/a:	0,201	h/a:	0,683		U_1:	UC_2:
c(p):	~~0,739~~	c(p):	~~0,354~~	h(comp)=	1,400 m	0,000 m
				kN/m²	*kN/m'*	*kN/m'*
Wind site:	Druck	c(p)=0,800	w(1) =	0,400	0,560	0,000
lateral surfaces:						
- for wind direction L_{KOM}:	Sog	c(p)=0,700	w(2,L) =	0,350	0,490	0,000
- for wind direction B_{KOM}:	Sog	c(p)=0,500	w(2,B) =	0,250	0,350	0,000
Roof:	Sog	c(p)=0,600	w(3) =	0,300		
wind on the leeward side:	Sog	c(p)=0,500	w(4) =	0,250	0,350	0,000

The simultaneous action of the snow ['s] and the wind load [w] (according to DIN 1055 T 4) is to consider the following load combinations: s + w / 2 or w + b / second.

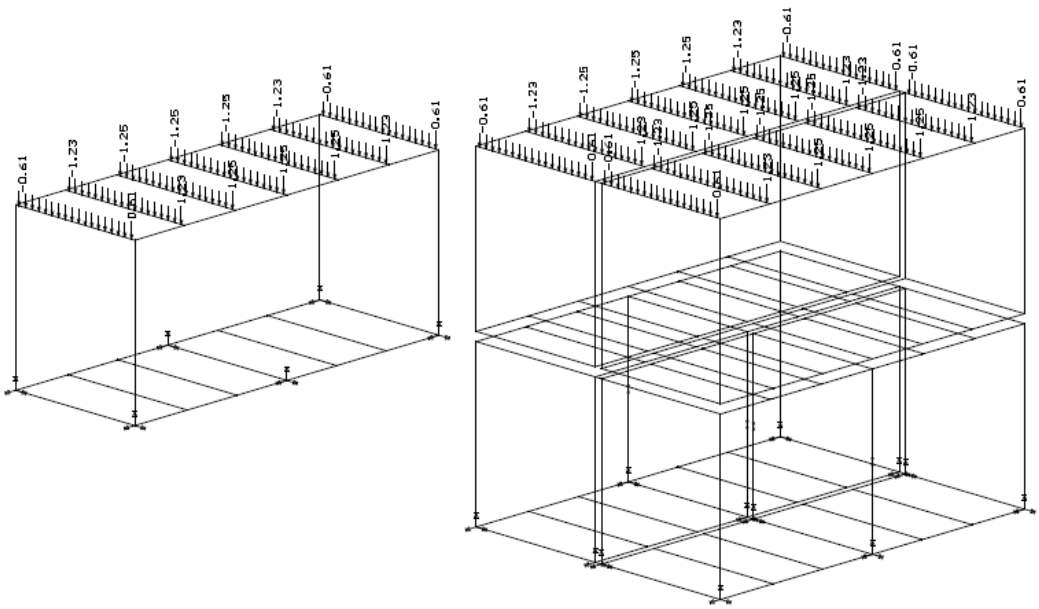
EXPOSURE PATTERNS OF CONTAINER:
L 1 - OWN WEIGHT (VALUES BY INTENDED PROGRAM)
L 2 - REMAINING ELASTIC (WEIGHT OF CONSTRUCTION KOMPLETATIONS)
$kN.m^{-1}$



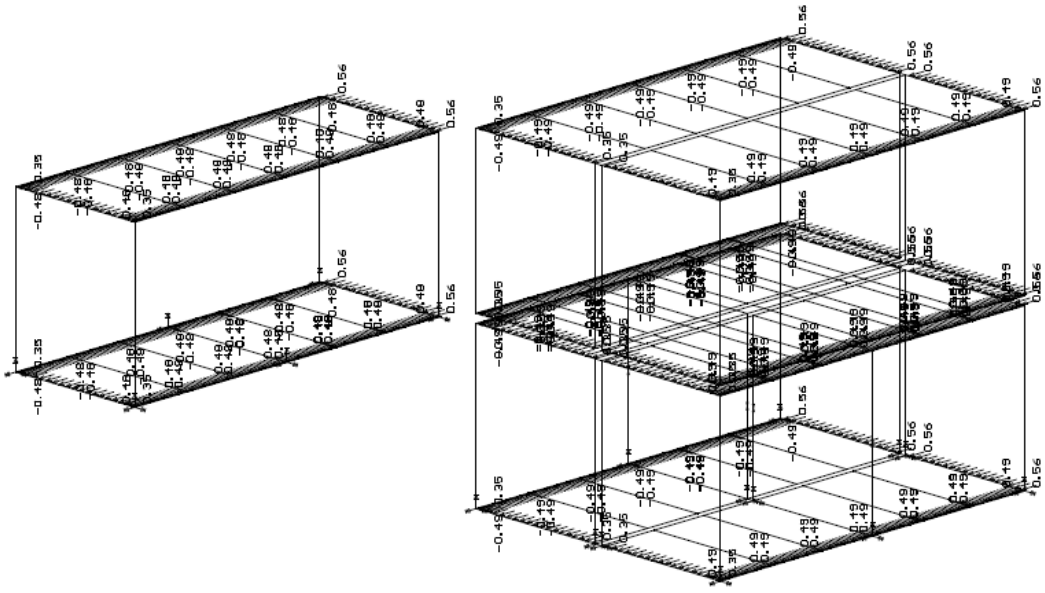
L 3 - PAYLOAD ON THE FLOOR
$kN.m^{-1}$



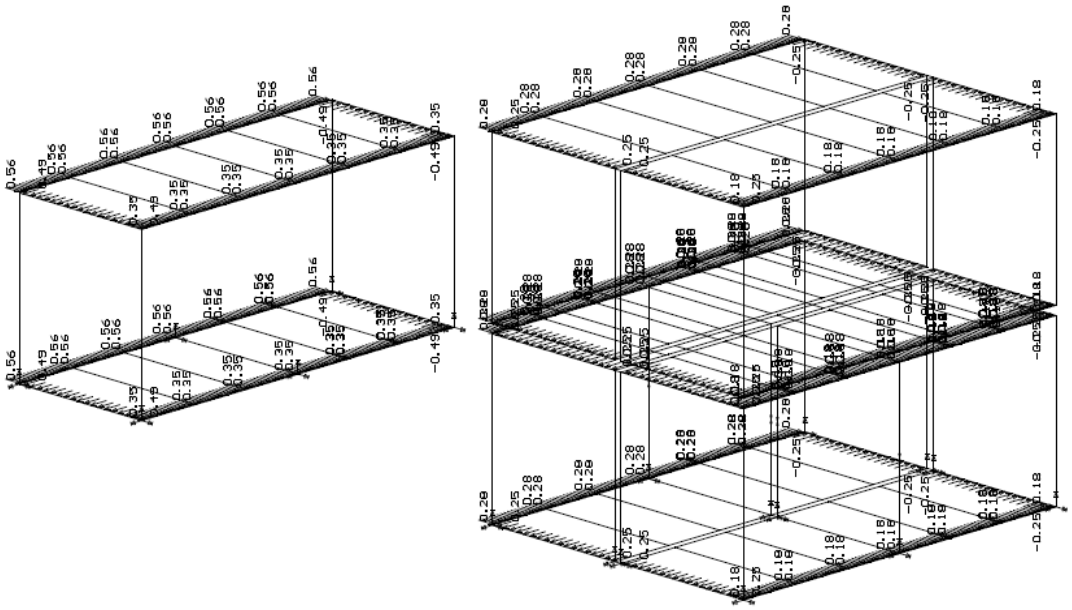
L 4 - ROOF USEFUL LOAD (SNOW)
$kN.m^{-1}$



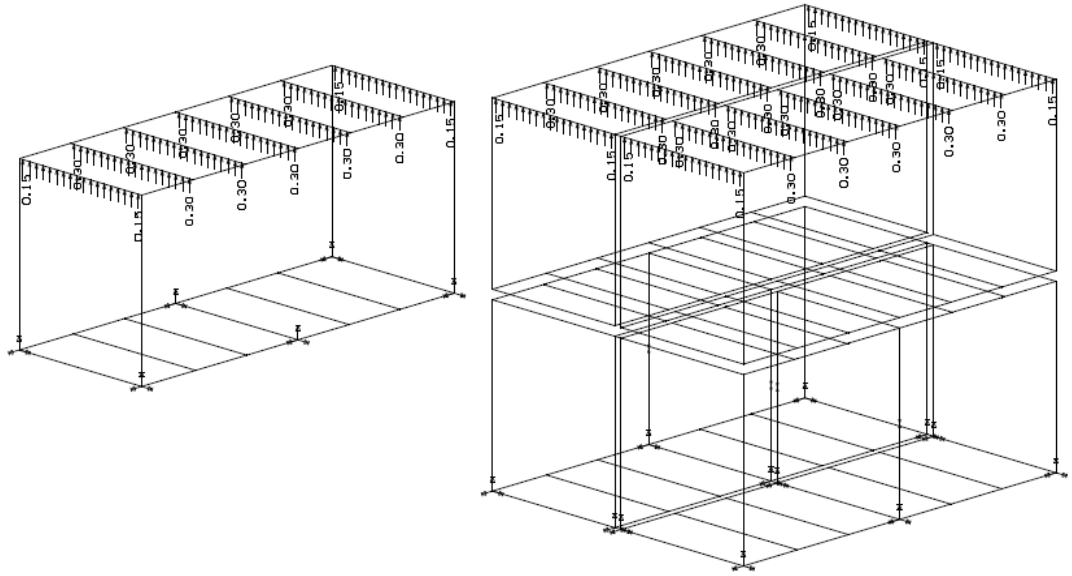
L 5 - WIND LOAD IN PREMISES LONGITUDINAL DIRECTION „+W(L)":
$kN.m^{-1}$



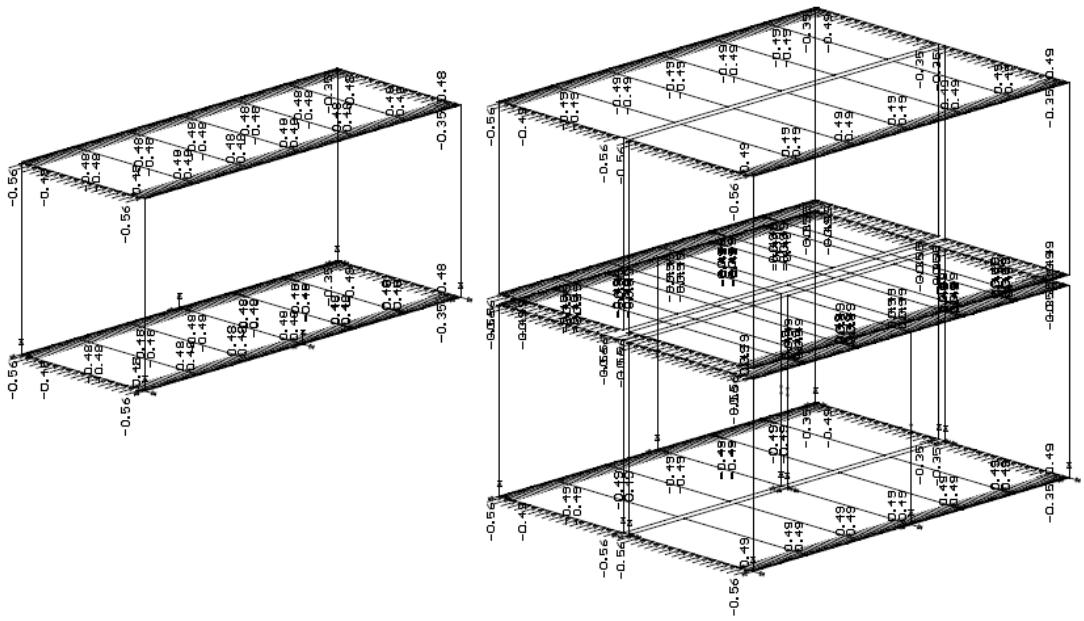
L 6 - WIND LOAD IN PREMISES CROSS DIRECTION „+W(Q)":
kN.m^{-1}



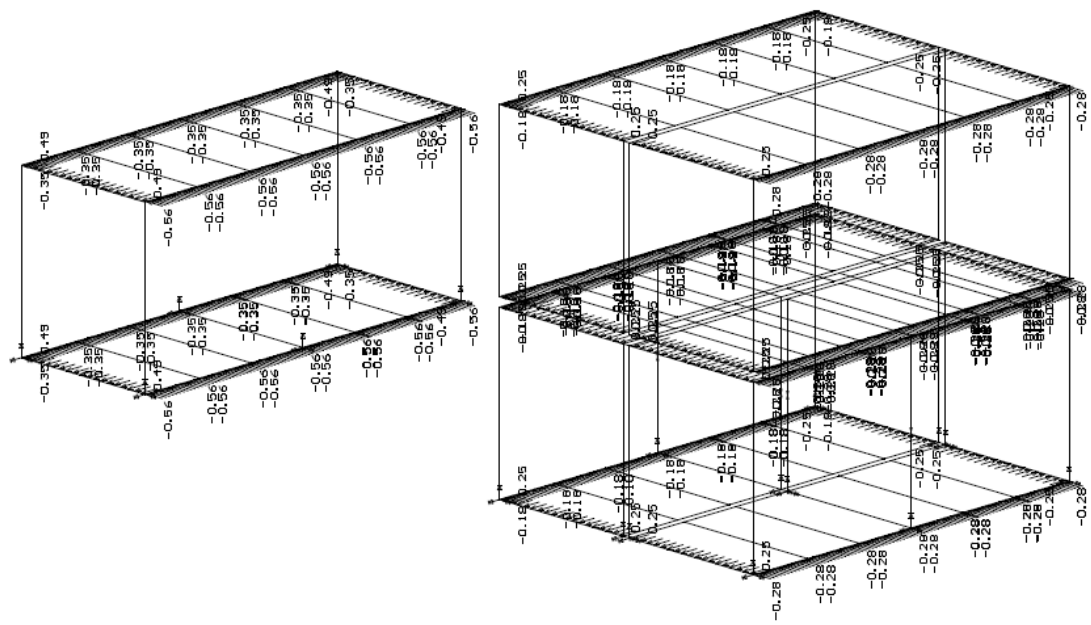
L 7 - WIND DRAWS ON PREMISES ROOF
kN.m^{-1}



L 8 - WIND LOAD IN PREMISES LONGITUDINAL DIRECTION „-W(L)":
$kN.m^{-1}$



L 9 - WIND LOAD IN PREMISES CROSS DIRECTION „-W(Q)":
$kN.m^{-1}$



ASSUMPTIONS FOR THE CALCULATION OF LAST:

Outline of loads:

L1	Dead weight (values determined using program)
L2	Other permanent loads (Weight of completing structures)
L3	Floor useful load
L4	Roof useful load (snow)
L5	Wind load in premises longitudinal direction "+W"
L6	Wind load in premises cross direction "+W"
L7	Wind draws on premises roof
L8	Wind load in premises longitudinal direction "-W"
L9	Wind load in premises cross direction "-W"

COMBINATION CALCULATION:

In calculation was utilization all possible combination load. Print-out combinations come out in tablet. Calculation combination was effected with below mentioned value coefficient.

γ_F =	1.35	for permanent loads
γ_F =	1.50	for useful loads
ψ =	1.00	for permanent loads
ψ =	1.00	for useful loads in combination
ψ =	0.90	for a higher quantity of useful loads in combination

ERECTION STAGE:

1.50*001	STATICS,LO_COND	1	DEAD-WEIGHT	0	0
1.50*002	STATICS,LO_COND	2	ERECTION STAGE	0	0

CONTINUOUS:

1.curve ~(~ 1~)
Factor combination: 1.000

1.35*001	STATICS,LOADCON	1	L1	0	0
1.35*002	STATICS,LOADCON	2	L2	0	0

2.curve ~(~ 2~)
Factor combination: 1.000

1.35*001	STATICS,LOADCON	1	L1	0	0
1.35*002	STATICS,LOADCON	2	L2	0	0
1.50*003	STATICS,LOADCON	3	L3	1	3

3.curve ~(~ 3~)
Factor combination: 1.000

1.35*001	STATICS,LOADCON	1	L1	0	0
1.35*002	STATICS,LOADCON	2	L2	0	0
1.50*004	STATICS,LOADCON	4	L4_SCHNEE	2	4

4.curve ~(~ 4~)
Factor combination: 1.000

1.35*001	STATICS,LOADCON	1	L1	0	0
1.35*002	STATICS,LOADCON	2	L2	0	0
1.35*003	STATICS,LOADCON	3	L3	1	3
1.35*004	STATICS,LOADCON	4	L4_SCHNEE	2	4

5.curve ~(~ 5~)
Factor combination: 1.000

1.35*001	STATICS,LOADCON	1	L1	0	0
1.35*002	STATICS,LOADCON	2	L2	0	0
1.50*005	STATICS,LOADCON	5	L5_+W(L)	2	5
1.50*007	STATICS,LOADCON	7	L7_W(DACH)	2	7

6.curve ~(~ 6~)
Factor combination: 1.000

1.35*001	STATICS,LOADCON	1	L1	0	0
1.35*002	STATICS,LOADCON	2	L2	0	0
1.50*006	STATICS,LOADCON	6	L6_+W(Q)	2	6
1.50*007	STATICS,LOADCON	7	L7_W(DACH)	2	7

7.curve ~(~ 7~)
Factor combination: 1.000

1.35*001	STATICS,LOADCON	1	L1	0	0
1.35*002	STATICS,LOADCON	2	L2	0	0
1.50*007	STATICS,LOADCON	7	L7_W(DACH)	2	7
1.50*008	STATICS,LOADCON	8	L8_-W(L)	1	8

8.curve ~(~ 8~)
Factor combination: 1.000

1.35*001	STATICS,LOADCON	1	L1	0	0
1.35*002	STATICS,LOADCON	2	L2	0	0
1.50*007	STATICS,LOADCON	7	L7_W(DACH)	2	7
1.50*009	STATICS,LOADCON	9	L9_-W(Q)	2	9

9.curve ~(~ 9~)
Factor combination: 1.000

1.35*001	STATICS,LOADCON	1	L1	0	0
1.35*002	STATICS,LOADCON	2	L2	0	0
1.35*003	STATICS,LOADCON	3	L3	1	3
1.35*005	STATICS,LOADCON	5	L5_+W(L)	2	5
1.35*007	STATICS,LOADCON	7	L7_W(DACH)	2	7

10.curve ~(~ 10~)
Factor combination: 1.000

1.35*001	STATICS,LOADCON	1	L1	0	0
1.35*002	STATICS,LOADCON	2	L2	0	0
1.35*003	STATICS,LOADCON	3	L3	1	3
1.35*006	STATICS,LOADCON	6	L6_+W(Q)	2	6
1.35*007	STATICS,LOADCON	7	L7_W(DACH)	2	7

11.curve ~(~ 11~)
Factor combination: 1.000

1.35*001	STATICS,LOADCON	1	L1	0	0
1.35*002	STATICS,LOADCON	2	L2	0	0
1.35*003	STATICS,LOADCON	3	L3	1	3
1.35*007	STATICS,LOADCON	7	L7_W(DACH)	2	7
1.35*008	STATICS,LOADCON	8	L8_-W(L)	1	8

12.curve ~(~ 12~)
Factor combination: 1.000

1.35*001	STATICS,LOADCON	1	L1	0	0
1.35*002	STATICS,LOADCON	2	L2	0	0
1.35*003	STATICS,LOADCON	3	L3	1	3
1.35*007	STATICS,LOADCON	7	L7_W(DACH)	2	7
1.35*009	STATICS,LOADCON	9	L9_-W(Q)	2	9

13.curve ~(~ 13~)
Factor combination: 1.000

1.35*001	STATICS,LOADCON	1	L1	0	0
1.35*002	STATICS,LOADCON	2	L2	0	0
1.35*004	STATICS,LOADCON	4	L4_SCHNEE	2	4
0.68*005	STATICS,LOADCON	5	L5_+W(L)	2	5
0.68*007	STATICS,LOADCON	7	L7_W(DACH)	2	7

14.curve ~(~ 14~)
Factor combination: 1.000

1.35*001	STATICS,LOADCON	1	L1	0	0
1.35*002	STATICS,LOADCON	2	L2	0	0
1.35*004	STATICS,LOADCON	4	L4_SCHNEE	2	4
0.68*006	STATICS,LOADCON	6	L6_+W(Q)	2	6
0.68*007	STATICS,LOADCON	7	L7_W(DACH)	2	7

15.curve ~(~ 15~)

```
Factor combination:   1.000
   1.35*001 STATICS,LOADCON  1         L1  0  0
   1.35*002 STATICS,LOADCON  2         L2  0  0
   1.35*004 STATICS,LOADCON  4  L4_SCHNEE  2  4
   0.68*007 STATICS,LOADCON  7 L7_W(DACH)  2  7
   0.68*008 STATICS,LOADCON  8   L8_-W(L)  1  8
 16.curve ~(~   16~)
Factor combination:   1.000
   1.35*001 STATICS,LOADCON  1         L1  0  0
   1.35*002 STATICS,LOADCON  2         L2  0  0
   1.35*004 STATICS,LOADCON  4  L4_SCHNEE  2  4
   0.68*007 STATICS,LOADCON  7 L7_W(DACH)  2  7
   0.68*009 STATICS,LOADCON  9   L9_-W(Q)  2  9
17.curve ~(~   17~)
Factor combination:   1.000
   1.35*001 STATICS,LOADCON  1         L1  0  0
   1.35*002 STATICS,LOADCON  2         L2  0  0
   1.35*003 STATICS,LOADCON  3         L3  1  3
   1.35*004 STATICS,LOADCON  4  L4_SCHNEE  2  4
   0.68*005 STATICS,LOADCON  5   L5_+W(L)  2  5
   0.68*007 STATICS,LOADCON  7 L7_W(DACH)  2  7
 18.curve ~(~   18~)
Factor combination:   1.000
   1.35*001 STATICS,LOADCON  1         L1  0  0
   1.35*002 STATICS,LOADCON  2         L2  0  0
   1.35*003 STATICS,LOADCON  3         L3  1  3
   1.35*004 STATICS,LOADCON  4  L4_SCHNEE  2  4
   0.68*006 STATICS,LOADCON  6   L6_+W(Q)  2  6
   0.68*007 STATICS,LOADCON  7 L7_W(DACH)  2  7
 19.curve ~(~   19~)
Factor combination:   1.000
   1.35*001 STATICS,LOADCON  1         L1  0  0
   1.35*002 STATICS,LOADCON  2         L2  0  0
   1.35*003 STATICS,LOADCON  3         L3  1  3
   1.35*004 STATICS,LOADCON  4  L4_SCHNEE  2  4
   0.68*007 STATICS,LOADCON  7 L7_W(DACH)  2  7
   0.68*008 STATICS,LOADCON  8   L8_-W(L)  1  8
 20.curve ~(~   20~)
Factor combination:   1.000
   1.35*001 STATICS,LOADCON  1         L1  0  0
   1.35*002 STATICS,LOADCON  2         L2  0  0
   1.35*003 STATICS,LOADCON  3         L3  1  3
   1.35*004 STATICS,LOADCON  4  L4_SCHNEE  2  4
   0.68*007 STATICS,LOADCON  7 L7_W(DACH)  2  7
   0.68*009 STATICS,LOADCON  9   L9_-W(Q)  2  9
 21.curve ~(~   21~)
Factor combination:   1.000
   1.35*001 STATICS,LOADCON  1         L1  0  0
   1.35*002 STATICS,LOADCON  2         L2  0  0
   0.68*004 STATICS,LOADCON  4  L4_SCHNEE  2  4
   1.35*005 STATICS,LOADCON  5   L5_+W(L)  2  5
   1.35*007 STATICS,LOADCON  7 L7_W(DACH)  2  7


22.curve ~(~   22~)
Factor combination:   1.000
   1.35*001 STATICS,LOADCON  1         L1  0  0
   1.35*002 STATICS,LOADCON  2         L2  0  0
   0.68*004 STATICS,LOADCON  4  L4_SCHNEE  2  4
   1.35*006 STATICS,LOADCON  6   L6_+W(Q)  2  6
   1.35*007 STATICS,LOADCON  7 L7_W(DACH)  2  7

 23.curve ~(~   23~)
Factor combination:   1.000
   1.35*001 STATICS,LOADCON  1         L1  0  0
   1.35*002 STATICS,LOADCON  2         L2  0  0
   0.68*004 STATICS,LOADCON  4  L4_SCHNEE  2  4
   1.35*007 STATICS,LOADCON  7 L7_W(DACH)  2  7
   1.35*008 STATICS,LOADCON  8   L8_-W(L)  1  8
 24.curve ~(~   24~)
Factor combination:   1.000
   1.35*001 STATICS,LOADCON  1         L1  0  0
   1.35*002 STATICS,LOADCON  2         L2  0  0
   0.68*004 STATICS,LOADCON  4  L4_SCHNEE  2  4
   1.35*007 STATICS,LOADCON  7 L7_W(DACH)  2  7
   1.35*009 STATICS,LOADCON  9   L9_-W(Q)  2  9
 25.curve ~(~   25~)
Factor combination:   1.000
   1.35*001 STATICS,LOADCON  1         L1  0  0
   1.35*002 STATICS,LOADCON  2         L2  0  0
   1.35*003 STATICS,LOADCON  3         L3  1  3
   0.68*004 STATICS,LOADCON  4  L4_SCHNEE  2  4
   1.35*005 STATICS,LOADCON  5   L5_+W(L)  2  5
   1.35*007 STATICS,LOADCON  7 L7_W(DACH)  2  7
 26.curve ~(~   26~)
Factor combination:   1.000
   1.35*001 STATICS,LOADCON  1         L1  0  0
   1.35*002 STATICS,LOADCON  2         L2  0  0
   1.35*003 STATICS,LOADCON  3         L3  1  3
   0.68*004 STATICS,LOADCON  4  L4_SCHNEE  2  4
   1.35*006 STATICS,LOADCON  6   L6_+W(Q)  2  6
   1.35*007 STATICS,LOADCON  7 L7_W(DACH)  2  7
 27.curve ~(~   27~)
Factor combination:   1.000
   1.35*001 STATICS,LOADCON  1         L1  0  0
   1.35*002 STATICS,LOADCON  2         L2  0  0
   1.35*003 STATICS,LOADCON  3         L3  1  3
   0.68*004 STATICS,LOADCON  4  L4_SCHNEE  2  4
   1.35*007 STATICS,LOADCON  7 L7_W(DACH)  2  7
   1.35*008 STATICS,LOADCON  8   L8_-W(L)  1  8
 28.curve ~(~   28~)
Factor combination:   1.000
   1.35*001 STATICS,LOADCON  1         L1  0  0
   1.35*002 STATICS,LOADCON  2         L2  0  0
   1.35*003 STATICS,LOADCON  3         L3  1  3
   0.68*004 STATICS,LOADCON  4  L4_SCHNEE  2  4
   1.35*007 STATICS,LOADCON  7 L7_W(DACH)  2  7
   1.35*009 STATICS,LOADCON  9   L9_-W(Q)  2  9
```

BERECHNUNGSERGEBNIS

THE EXTREME VALUES FOR INDIVIDUAL CROSS

The combinations are edited Article condition in the Distribution of admission values.

FÜR DIE EXTREMWERTE IST GILTIG:

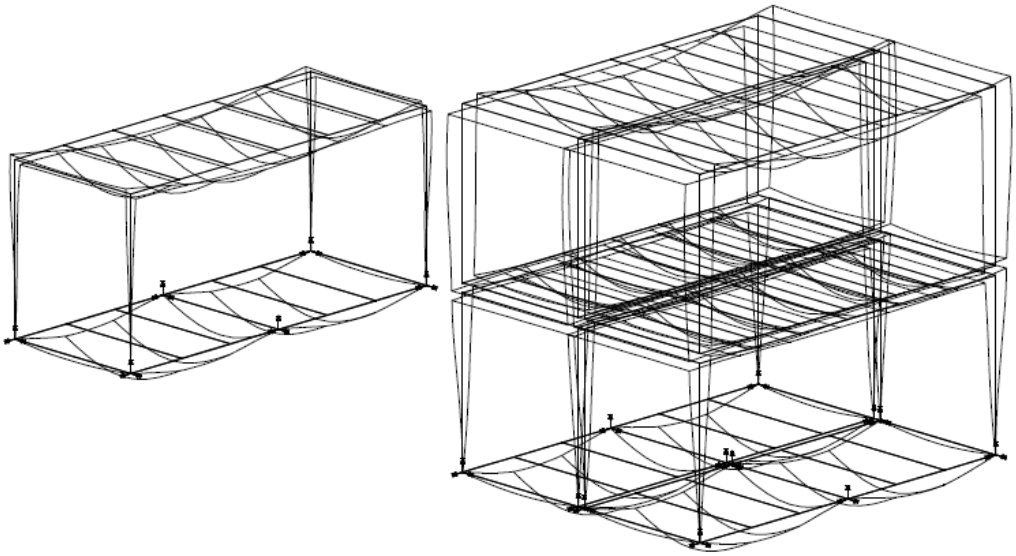
Using the program are all load combinations calculated in accordance with the breakdown showing the extreme values and individual sizes from across the combination of scope.

The maximum shift for the positive and determined as well as for the negative branch of the envelope curve.

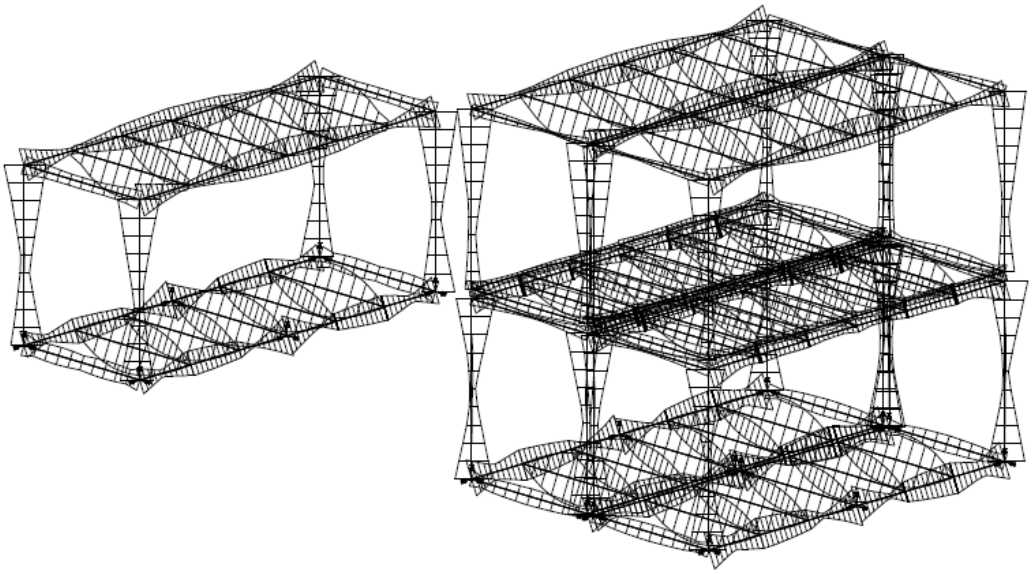
The maxims of the nearest sizes are determined from the positive branch of the envelope curve.

The minima of the nearest sizes are determined from the negative branch of the envelope curve.

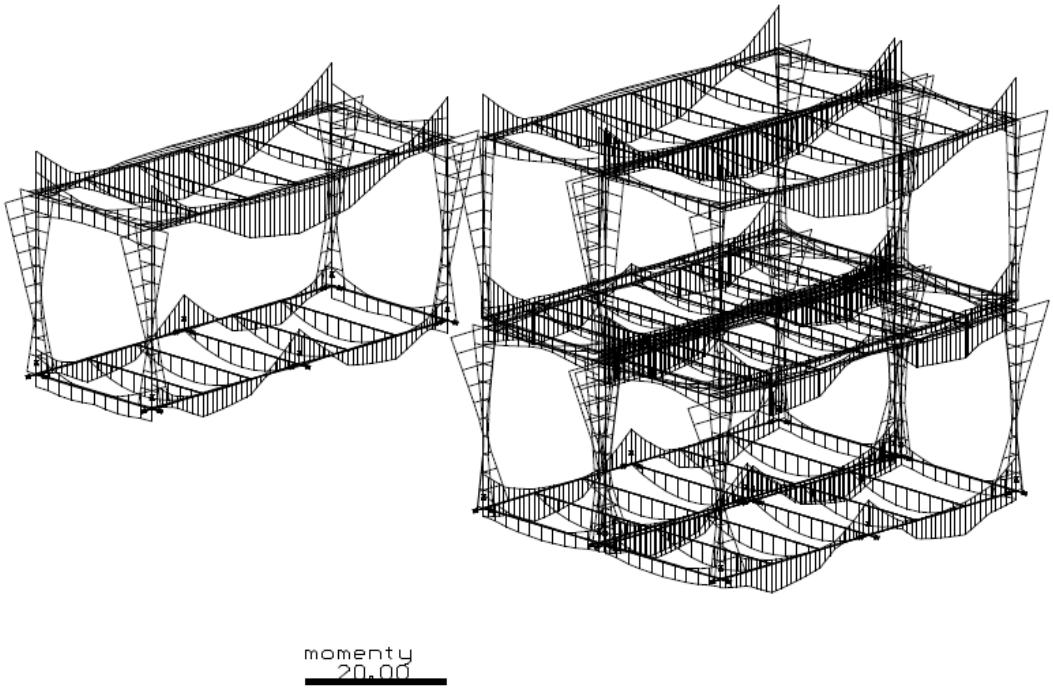
RESULTS - DEFORMATION:
[m]



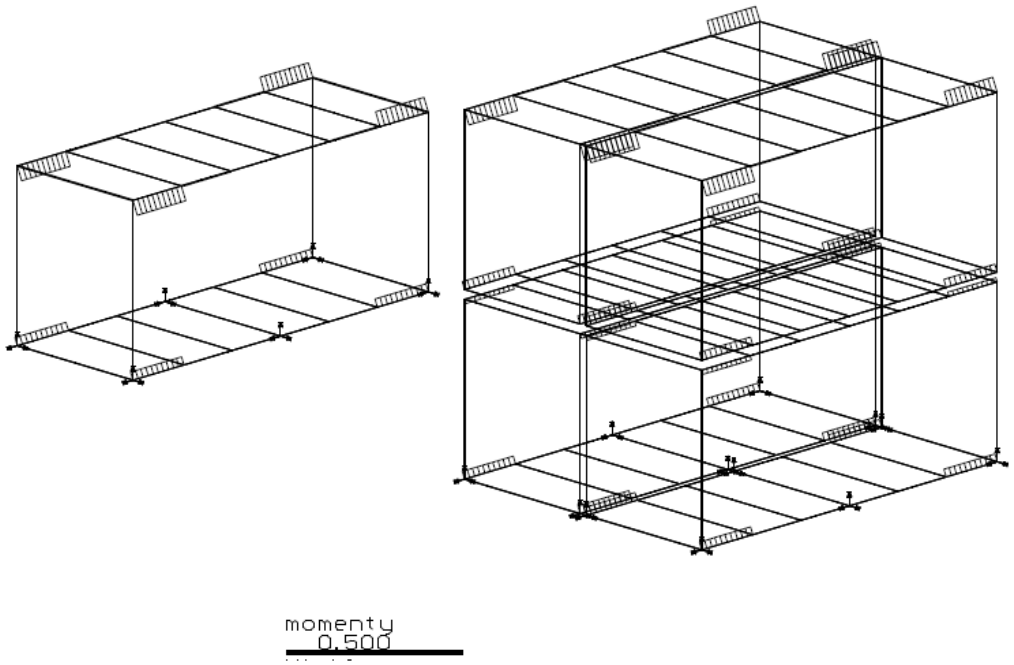
RESULTS - VOLTAGE":
[kPa]



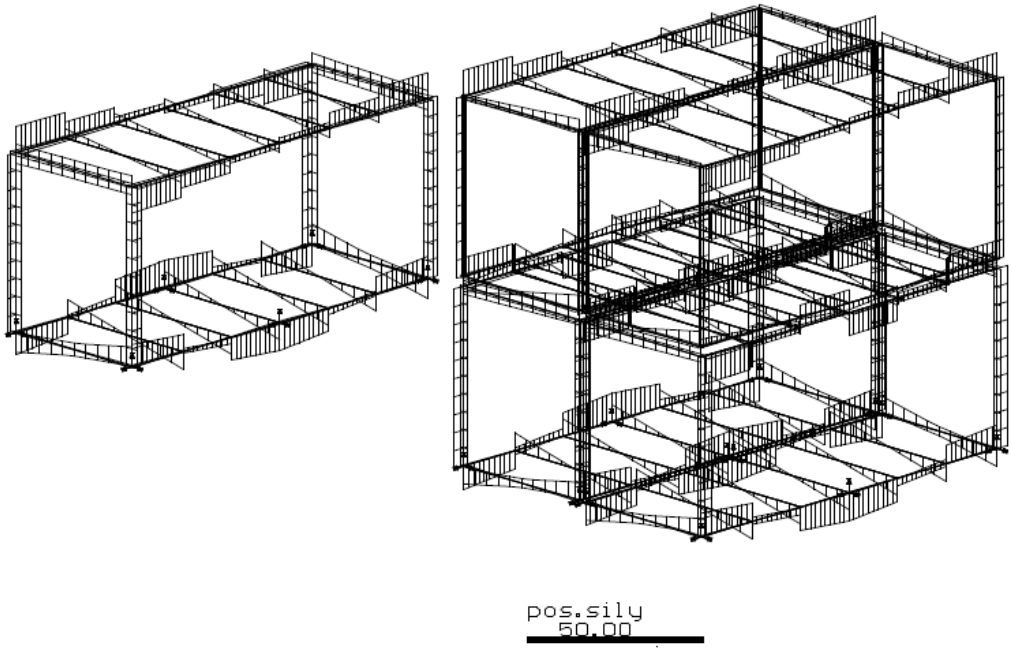
RESULTS - BIEGUNGSMOMENT:
[kN.m]



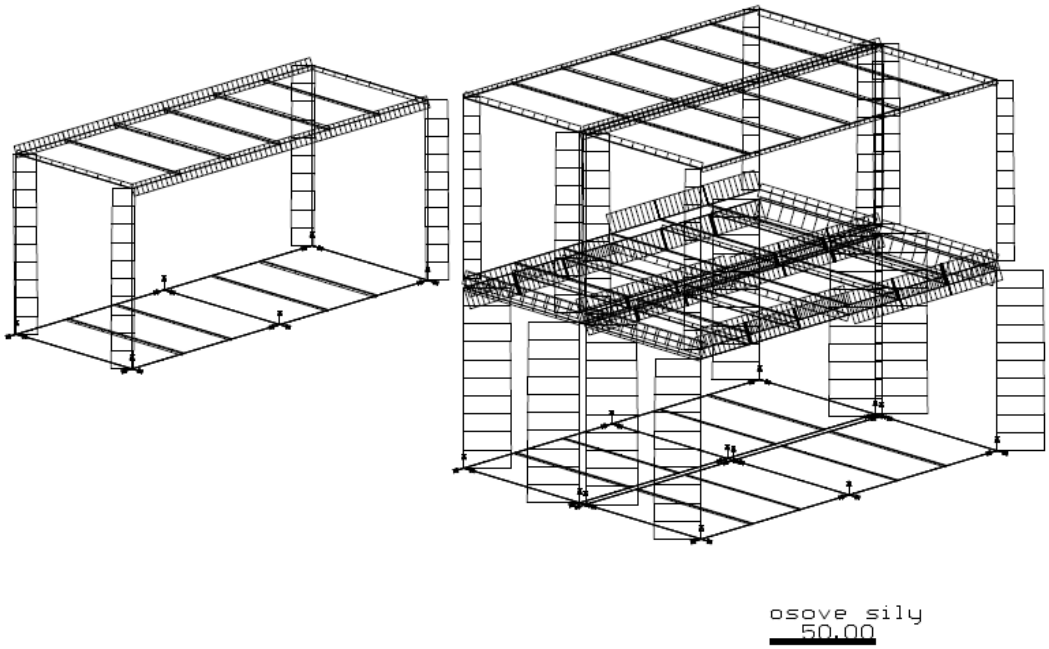
RESULTS - TORQUE:
[kN.m]



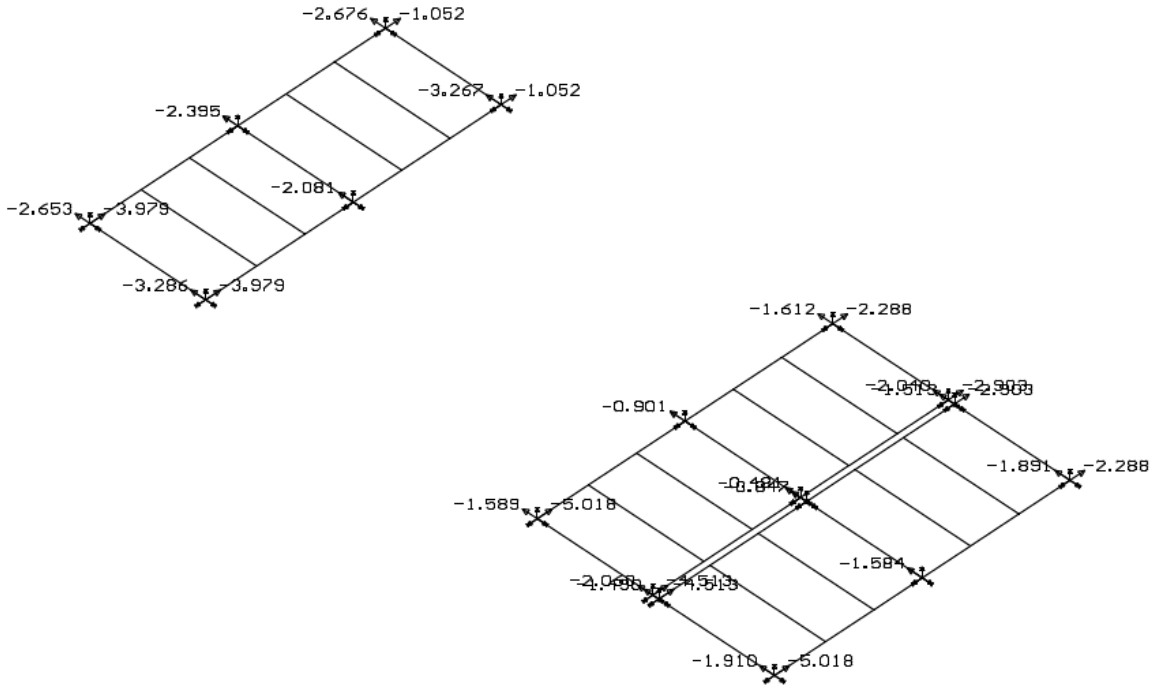
RESULTS - THRUST FORCE:
[kN]



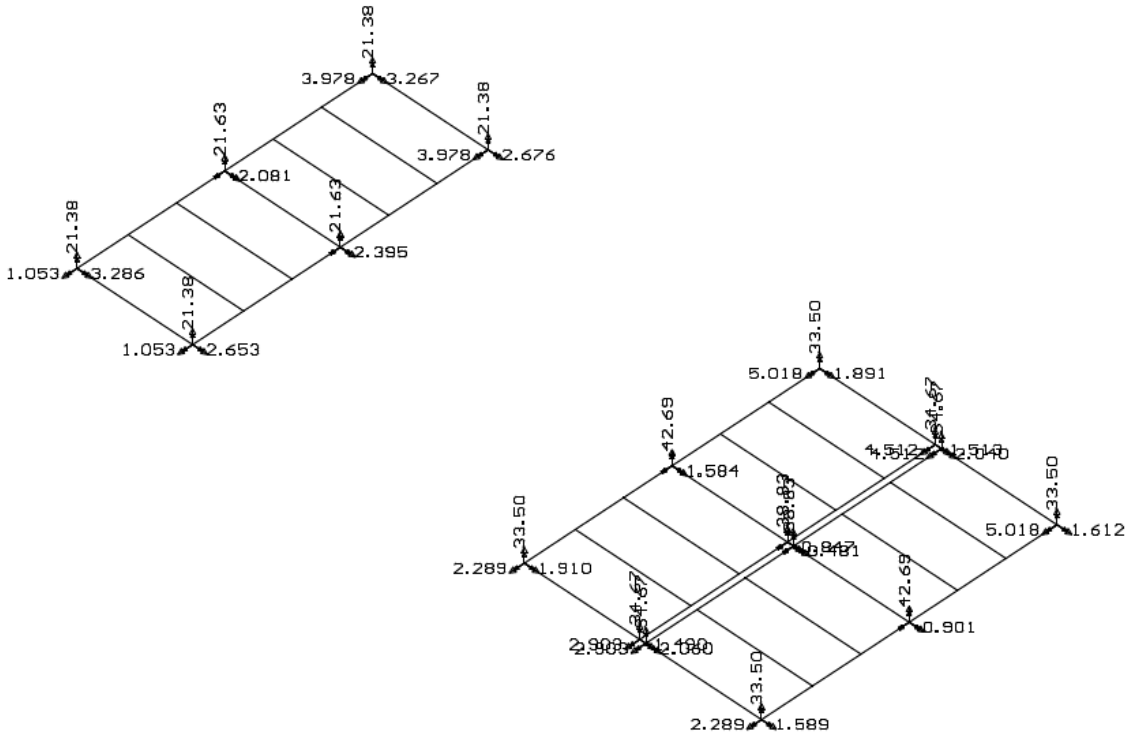
RESULTS - NORMAL FORCE:
[kN]



RESULTS – SUPPORT REACTION MIN:
[kN]



RESULTS – SUPPORT REACTION MAX:
[kN]



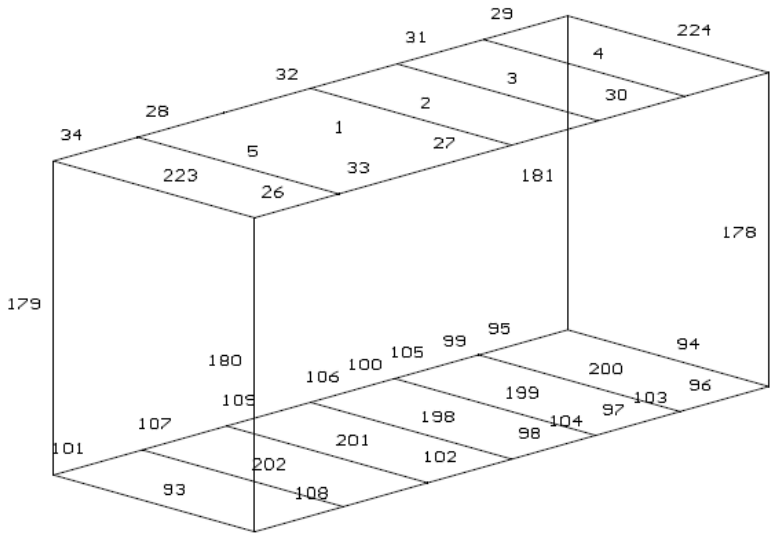
DIMENSIONIERRUNG DER STAHLDURCHSCHNITTEN NACH EUROCODE 3
INFORMATION ABOUT EXERCISE
Drawing task: : PE_STA
Type of mission : Space
Folder with Data: : D:\FEATDATA\ PE_STA\

Type of curve: +/-
Each KZS: 28
Komplex der Auggabe: PE_STA.OBL,
Complex calculation: PE_STA.%01

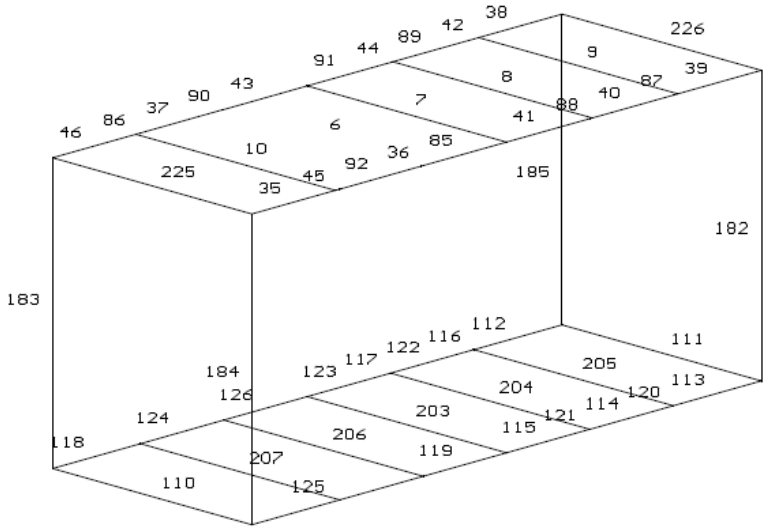
Coefficient: $\gamma_{m,0} = 1,1$
$\gamma_{m,1} = 1,1$
$\gamma_{m,2} = 1,25$

Prurez	- Cross-section
cislo dilce	- Part Number
poradnice	- Coordinate
Trida prurezu	- Average Class
spoctena	- Calculated
POSUDEK	- Evaluation
Tlakova unosnost	- Print investigation
Prut na tlak vyhovi	- Rod corresponds
Smykova unosnost prurezu	- thrust investigation
Momentove unosnosti prurezu	- Moments investigation
Stabilita dilce	- Stabstabilität
Interakce	- Interaction
Podmínka splnena	- Die Bedingug erfüllt
Prurez vyhovi	- Teil entspricht
Dilec vyhovi	- Item corresponds

PART SIZING:
C_0



C_1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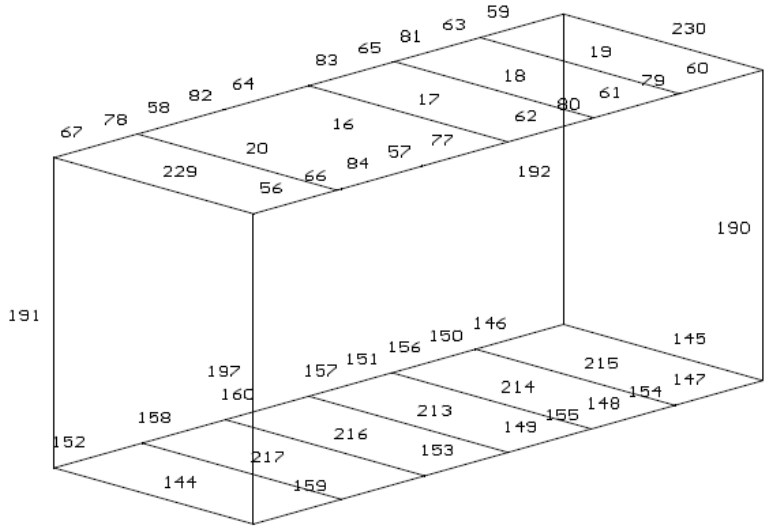
C_2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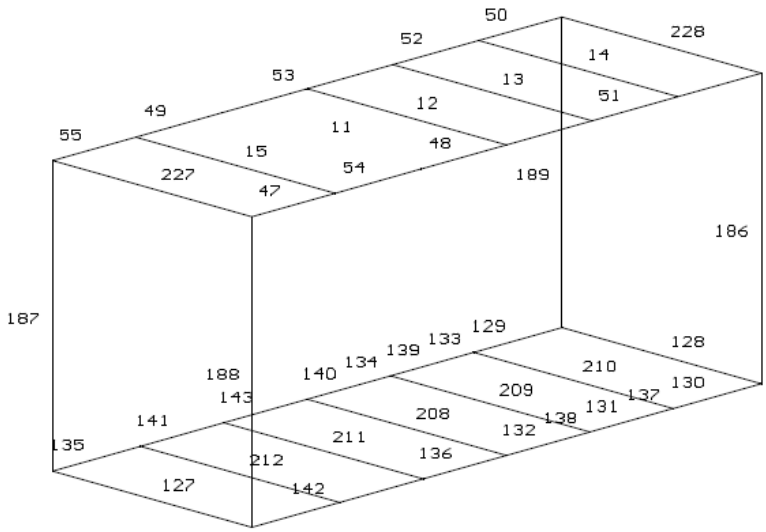
C_3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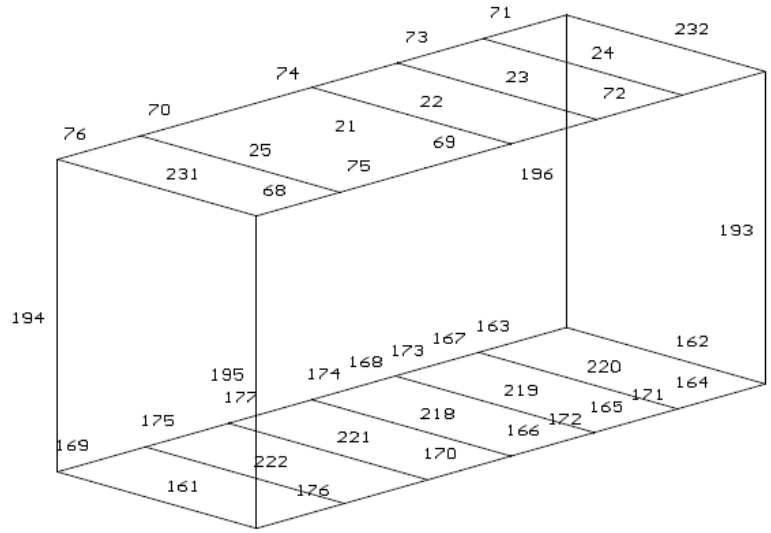
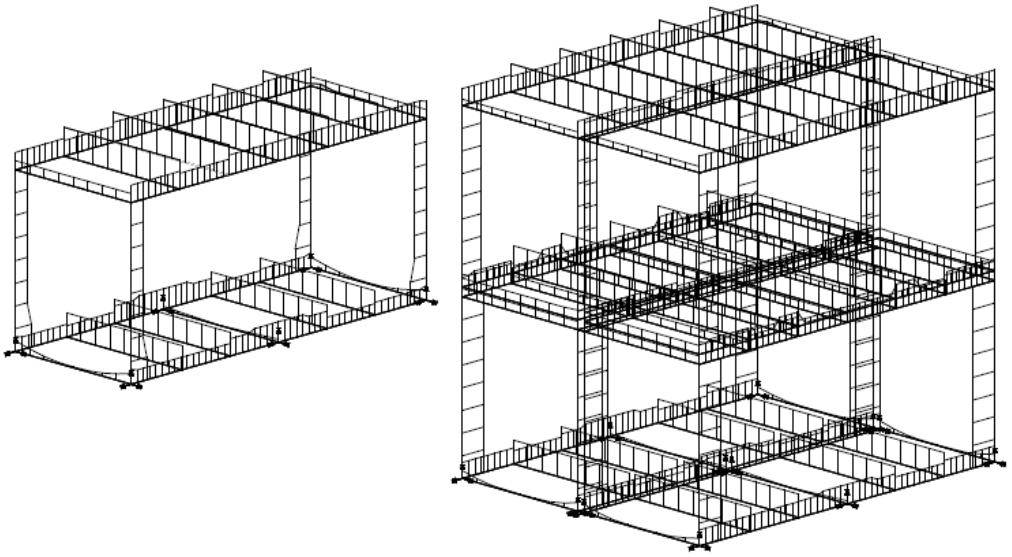
C_4



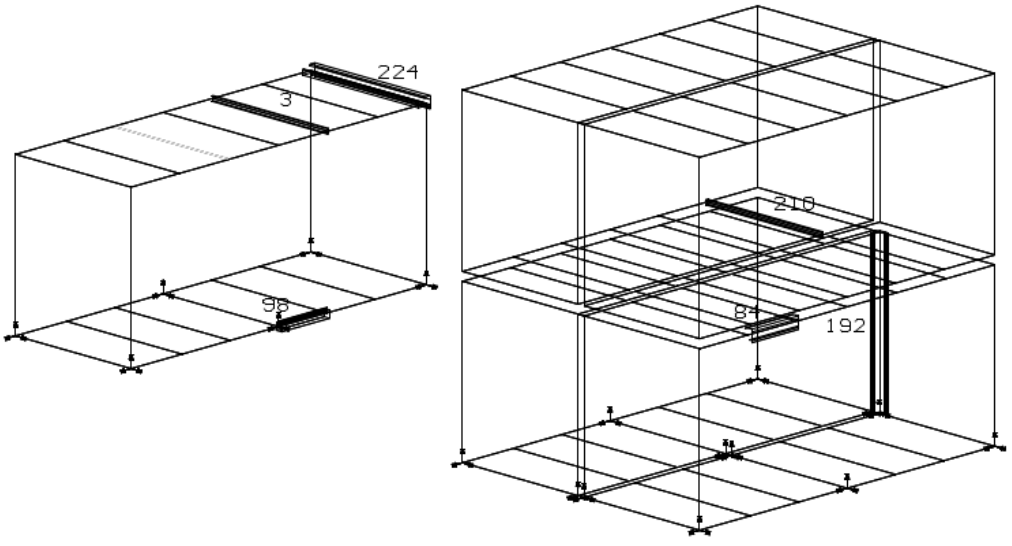
ALL - CAPACITY UTILIZATION (σ / σ_{RD} ≤ 1):

C_0 C_3 C_4



C_1 C_2

PART WITH EXTREME



EXTREMTAFFEL:

TABLES OF EXTREME VALUES - SECTION: Q1_DECK

	MINIMUM			MAXIMUM		
Parameter:	Wert:	Teil:	Lage:	Wert:	Teil:	Lage:	
bending moment My	-1.625	12	1.292	0.046	14	0.0
bending moment Mz	-0.042	4	2.326	0.042	4	2.326
moment of torsion Mx	-4.675E-5	24	0.0	4.675E-5	14	0.0
normal force N	-1.776	8	0.0	3.265	17	0.0
shear force Qz	-2.858	11	0.0	2.858	21	2.326
shear force Qy	2.833E-5	2	0.0	-2.834E-5	2	0.0
abs shift ux	-0.010	21	0.0	0.010	14	1.809
abs shift uy	-0.008	2	1.292	0.008	2	2.326
abs shift uz	0.0	1	0.0	-0.001	7	0.0
abs tot.shift uc	0.006	5	0.0	0.020	22	1.034
rel shift rx	-2.062E-4	14	0.517	2.066E-4	15	0.517
rel shift ry	-3.365E-6	1	1.034	3.366E-6	1	1.034
rel shift rz	-4.322E-4	9	0.258	0.0	1	0.0
rel tot.shift rc	0.0	1	0.0	0.008	2	1.292
utilization	0.232	9	0.0	0.876	3	0.0

TABLES OF EXTREME VALUES - SECTION: Q2_OLRA

	MINIMUM			MAXIMUM		
Parameter:	Wert:	Teil:	Lage:	Wert:	Teil:	Lage:	
bending moment My	-7.282	27	1.000	8.121	60	0.973
bending moment Mz	-2.737	60	0.973	2.500	30	0.0
moment of torsion Mx	-0.049	71	0.0	0.049	51	2.000
normal force N	-3.834	51	0.0	10.407	62	0.0
shear force Qz	-8.377	38	0.0	8.377	60	0.973
shear force Qy	0.0	38	0.243	0.0	36	0.0
abs shift ux	-0.010	75	0.750	0.010	50	0.0
abs shift uy	-0.008	32	1.000	0.008	32	1.000
abs shift uz	0.0	27	0.0	-4.522E-5	34	0.973
abs tot.shift uc	0.005	34	0.973	0.014	69	1.000
rel shift rx	-2.128E-6	74	1.000	2.129E-6	74	1.000
rel shift ry	-5.082E-4	32	1.500	5.085E-4	32	1.500
rel shift rz	-1.140E-4	60	0.486	1.648E-4	67	0.487
rel tot.shift rc	0.0	26	0.0	0.001	30	1.000
utilization	0.257	82	0.0	0.712	84	0.710

TABLES OF EXTREME VALUES - SECTION: Q3_UL&QRA

	MINIMUM			MAXIMUM		
Parameter:	Wert:	Teil:	Lage:	Wert:	Teil:	Lage:	
bending moment My	-4.234	125	0.973	5.181	102	0.0
bending moment Mz	-1.170	142	2.000	1.170	127	2.326
moment of torsion Mx	-0.024	163	0.0	0.024	130	0.0
normal force N	-9.432	176	0.973	3.561	161	0.0
shear force Qz	-8.477	119	0.0	8.477	160	0.973
shear force Qy	0.028	156	0.968	-0.028	114	0.0
abs shift ux	-0.008	162	1.292	0.008	128	1.551
abs shift uy	-0.004	140	0.968	0.005	140	0.968
abs shift uz	0.0	93	0.0	0.0	93	0.0
abs tot.shift uc	0.0	93	0.0	0.009	170	0.243
rel shift rx	-2.929E-4	94	1.292	3.396E-4	128	1.551
rel shift ry	-7.230E-5	159	0.750	1.256E-4	159	1.250
rel shift rz	-1.180E-4	142	1.750	1.213E-4	142	1.750
rel tot.shift rc	0.0	93	0.0	0.002	125	0.973
utilization	0.017	110	1.034	0.605	98	0.0

TABLES OF EXTREME VALUES - SECTION: Q4_ECKS

	MINIMUM			MAXIMUM		
Parameter:	Wert:	Teil:	Lage:	Wert:	Teil:	Lage:
bending moment My	-6.629	186	2.674	5.014	185	2.674
bending moment Mz	-3.704	186	2.674	3.228	178	2.674
moment of torsion Mx	-0.001	196	0.0	0.001	186	0.0
normal force N	-24.825	192	2.674	1.373	181	0.0
shear force Qz	-4.335	190	0.0	4.335	185	0.0
shear force Qy	0.643	193	0.0	-0.643	189	0.0
abs shift ux	-0.010	193	2.674	0.011	187	2.188
abs shift uy	-0.004	178	2.674	0.005	178	2.674
abs shift uz	0.0	178	0.0	0.0	178	0.0
abs tot.shift uc	0.0	178	0.0	0.011	195	0.486
rel shift rx	-6.477E-4	180	1.459	0.001	195	0.972
rel shift ry	-5.917E-4	181	0.972	6.458E-4	179	1.459
rel shift rz	-0.0	180	0.972	2.997E-6	188	0.729
rel tot.shift rc	0.0	178	0.0	0.001	189	0.972
utilization	0.468	180	0.0	0.954	192	0.0

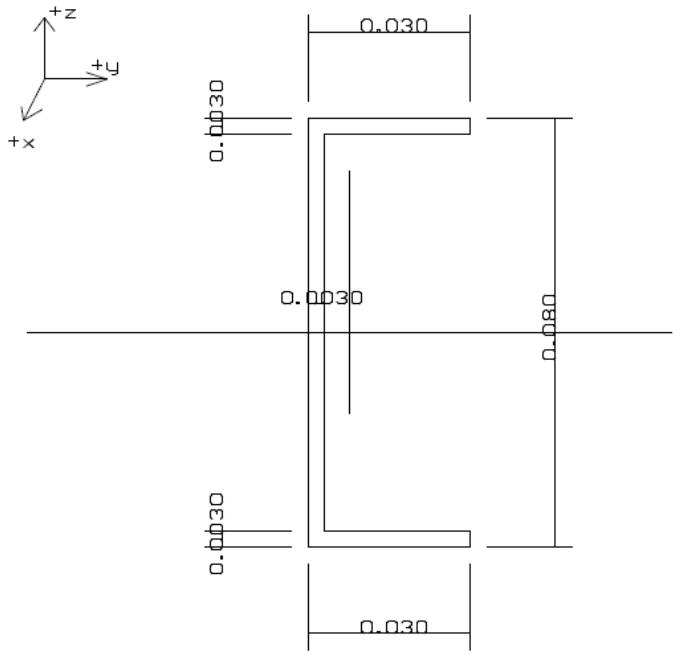
TABLES OF EXTREME VALUES - SECTION: Q5_FUSS

	MINIMUM			MAXIMUM		
Parameter:	Wert:	Teil:	Lage:	Wert:	Teil:	Lage:
bending moment My	-2.956	207	1.034	0.025	220	2.326
bending moment Mz	-0.076	220	0.0	0.076	220	2.326
moment of torsion Mx	-9.002E-5	210	0.0	9.002E-5	220	0.0
normal force N	-0.391	202	0.0	1.005	200	0.0
shear force Qz	-5.240	210	0.0	5.240	220	2.326
shear force Qy	2.309E-5	203	0.0	-2.309E-5	213	0.0
abs shift ux	-0.008	218	0.0	0.008	220	0.517
abs shift uy	-0.004	208	2.326	0.005	208	2.326
abs shift uz	0.0	198	0.0	0.0	198	0.0
abs tot.shift uc	0.0	198	0.0	0.012	221	1.034
rel shift rx	-2.054E-4	212	1.809	2.403E-4	220	0.517
rel shift ry	-8.813E-8	202	1.034	9.576E-7	205	1.292
rel shift rz	-0.001	211	1.034	0.0	198	0.0
rel tot.shift rc	0.0	198	0.0	0.007	200	1.034
utilization	0.010	203	1.292	0.791	210	0.0

TABLES OF EXTREME VALUES - SECTION: Q6_OQRA

	MINIMUM			MAXIMUM		
Parameter:	Wert:	Teil:	Lage:	Wert:	Teil:	Lage:
bending moment My	-3.392	224	2.326	3.224	224	0.0
bending moment Mz	-2.737	230	0.0	1.564	230	2.326
moment of torsion Mx	-0.002	224	0.0	0.002	224	0.0
normal force N	-1.246	230	0.0	7.601	226	0.0
shear force Qz	-3.443	231	0.0	3.443	227	2.326
shear force Qy	0.039	225	0.517	-0.039	229	1.809
abs shift ux	-0.010	232	0.0	0.010	228	1.551
abs shift uy	-0.004	224	0.0	0.005	224	0.0
abs shift uz	0.0	225	0.0	-4.522E-5	223	0.0
abs tot.shift uc	0.005	223	1.034	0.011	227	0.775
rel shift rx	-4.768E-5	227	0.775	1.484E-4	224	1.034
rel shift ry	-1.210E-6	223	1.034	1.210E-6	223	1.034
rel shift rz	0.0	223	0.0	0.0	223	0.0
rel tot.shift rc	0.0	223	0.0	6.636E-4	223	1.034
utilization	0.186	232	0.0	0.270	224	0.0

ASSESSMENT - SECTIONS: Q_1

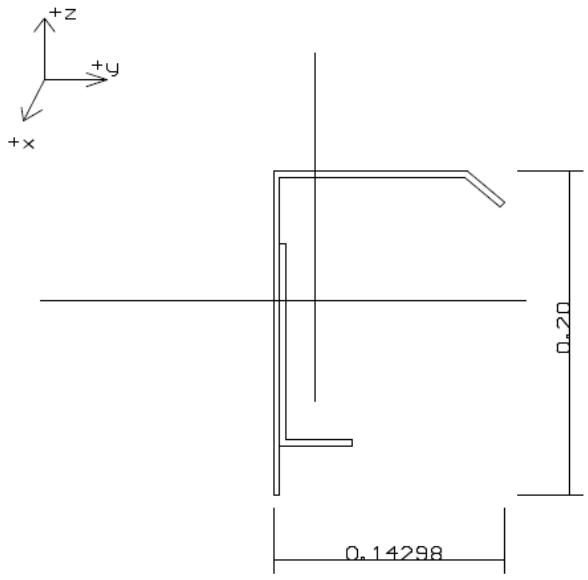



```
POSUDEK
Tlakova unosnost N(b.Rd)=    85.88 kN
 0.01 < 1  -> Prut na tlak vyhovi

Smykova unosnost prurezu:
V(z.Rd)=    28.81 kN
 0.10 < 1 -> Prurez vyhovi
Smykova unosnost prurezu:
V(y.Rd)=    19.44 kN
 0.00 < 1 -> Prurez vyhovi
Momentove unosnosti prurezu
M(c.y.Rd)=     1.97 kNm
M(c.z.Rd)=     0.90 kNm
Interakce:   0.05 < 1 -> Prurez vyhovi

Stabilita dilce:
 0.88 < 1 -> Podminka splnena

Dilec VYHOVI
```

Prurez	- Section
cislo dilce	- Part number
poradnice	- Ordinate
Trida prurezu	- Section class
Spoctena	- Calculated
POSUDEK	- ASSESSMENT
Tlakova unosnost	- Pressure load bearing capacity
Prut na tlak vyhovi	- Rod pressure strength satisfactory
Smykova unosnost prurezu	- Section shear strength
momentove unosnosti prurezu	- Section moment load bearing capacity
Stabilita dilce	- Part stability
Interakce	- Interaction
Podminka splnena	- condition complied with
Prurez vyhovi	- Section satisfactory
Dilec vyhovi	- Part satisfactory

```
Prurez: Q1_DECK       cislo dilce=     3     N(Sd) =    0.6 kN
A    =       402.0 mm2poradnice=     0.0 m  N(Ob) =   -1.2 kN
Iy   =  3.68000E+5 mm4Ly =      0.0 m       My(Sd)= 2.3E-3 kNm
Iz   =     31783.0 mm4Lz =      0.0 m       My(Ob)=-2.5E-4 kNm
It   =      1600.0 mm4Kat. pripoje: neni C  Mz(Sd)= 3.3E-2 kNm
Iw   = 9.24936E+7 mm6                      Mz(Ob)=-3.3E-2 kNm
Wply=      9200.0 mm3                      Vz(Ob)=   -2.9 kN
Wplz=      1415.1 mm3                      Vy(Sd)= 2.9E-2 kN
Trida prurezu= 3                           Vy(Ob)=-2.9E-2 kN
                                           Mx(Sd)= 3.5E-5 kNm
                                           Mx(Ob)=-3.5E-5 kNm
                                           N max =   -1.2 kN
                                           My max=   -1.6 kNm
                                           Mz max= 3.3E-2 kNm
```

ASSESSMENT - SECTIONS: Q_2

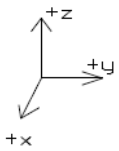


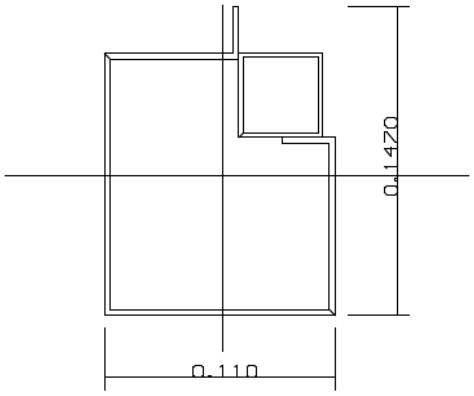

```
POSUDEK

Smykova unosnost prurezu:
V(z.Rd)=   190.33 kN
 0.01 < 1 -> Prurez vyhovi
Smykova unosnost prurezu:
V(y.Rd)=    32.96 kN
 0.03 < 1 -> Prurez vyhovi
Momentove unosnosti prurezu
M(c.y.Rd)=    17.42 kNm
M(c.z.Rd)=     3.56 kNm
Interakce:   0.42 < 1 -> Prurez vyhovi

Stabilita dilce:
 0.71 < 1 -> Podminka splnena

Dilec VYHOVI
```

Prurez	- Section
cislo dilce	- Part number
poradnice	- Ordinate
Trida prurezu	- Section class
Spoctena	- Calculated
POSUDEK	- ASSESSMENT
Tlakova unosnost	- Pressure load bearing capacity
Prut na tlak vyhovi	- Rod pressure strength satisfactory
Smykova unosnost prurezu	- Section shear strength
momentove unosnosti prurezu	- Section moment load bearing capacity
Stabilita dilce	- Part stability
Interakce	- Interaction
Podminka splnena	- condition complied with
Prurez vyhovi	- Section satisfactory
Dilec vyhovi	- Part satisfactory

```
Prurez: Q2_OLRA       cislo dilce=    84     N(Sd) =   10.4 kN
A    =      2043.0 mm2poradnice=   0.710 m  My(Sd)=    0.2 kNm
Iy   =  9.77800E+6 mm4Kat. pripoje: neni C  My(Ob)=   -4.3 kNm
Iz   =  1.94800E+6 mm4                      Mz(Sd)=    1.5 kNm
It   =     37200.0 mm4                      Vz(Sd)=    1.2 kNm
Iw   = 2.5016E+10 mm6                      Vz(Ob)=   -1.4 kNm
Wply=  1.22149E+5 mm3                      Vy(Sd)=    0.1 kNm
Wplz=     16649.6 mm3                      Vy(Ob)=   -0.9 kNm
Trida prurezu= 3                           Mx(Ob)=-8.2E-4 kNm
                                           N max =   10.4 kN
                                           My max=   -4.8 kNm
                                           Mz max=    1.5 kNm
```

ASSESSMENT - SECTIONS: Q_3



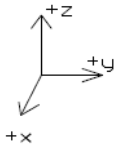


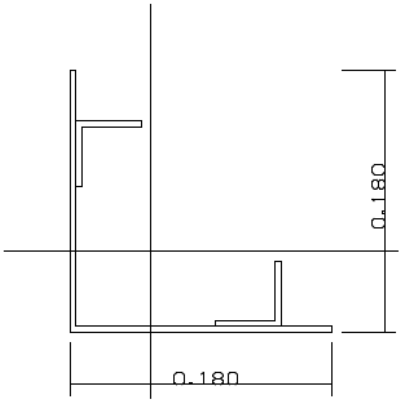

```
POSUDEK
Tlakova unosnost N(b.Rd)=  333.49 kN
 0.00 < 1  -> Prut na tlak vyhovi

Smykova unosnost prurezu:
V(z.Rd)=  102.78 kN
 0.08 < 1 -> Prurez vyhovi
Smykova unosnost prurezu:
V(y.Rd)=   91.01 kN
 0.01 < 1 -> Prurez vyhovi
Momentove unosnosti prurezu
M(c.y.Rd)=    9.48 kNm
M(c.z.Rd)=   10.17 kNm
Interakce:  0.61 < 1 -> Prurez vyhovi

Stabilita dilce:
 0.61 < 1 -> Podminka splnena

Dilec VYHOVI
```

Prurez	- Section
cislo dilce	- Part number
poradnice	- Ordinate
Trida prurezu	- Section class
Spoctena	- Calculated
POSUDEK	- ASSESSMENT
Tlakova unosnost	- Pressure load bearing capacity
Prut na tlak vyhovi	- Rod pressure strength satisfactory
Smykova unosnost prurezu	- Section shear strength
momentove unosnosti prurezu	- Section moment load bearing capacity
Stabilita dilce	- Part stability
Interakce	- Interaction
Podminka splnena	- condition complied with
Prurez vyhovi	- Section satisfactory
Dilec vyhovi	- Part satisfactory

```
Prurez: Q3_UL80RA     cislo dilce=    98      N(Sd) = 3.9E-3 kN
A    =       1561.0 mm2poradnice=      0.0 m  N(Ob) =-3.9E-3 kN
Iy   =  3.57500E+6 mm4Ly =       0.0 m        My(Sd)=    5.2 kNm
Iz   =  2.66800E+6 mm4Lz =       0.0 m        Mz(Sd)=    0.6 kNm
It   =      23800.0 mm4Kat. pripoje: neni C   Mz(Ob)=   -0.6 kNm
Iw   =  1.5501E+10 mm6                       Vz(Ob)=   -8.3 kN
Wply=      44387.9 mm3                       Vy(Sd)=    1.2 kN
Wplz=      49462.4 mm3                       Vy(Ob)=   -1.0 kN
Trida prurezu= 3                             Mx(Sd)= 1.6E-4 kNm
                                             Mx(Ob)=-1.6E-4 kNm
                                             N max =-3.9E-3 kN
                                             My max=    5.2 kNm
                                             Mz max=    0.6 kNm
```

ASSESSMENT - SECTIONS: Q_4



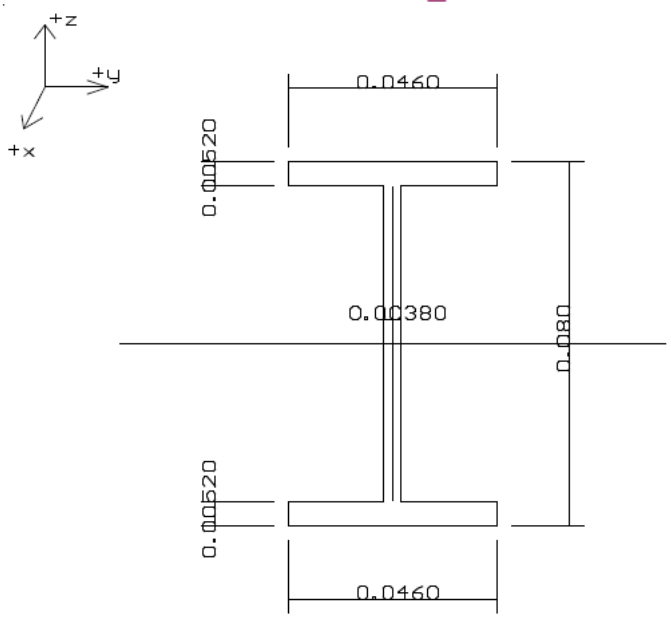



```
POSUDEK

Smykova unosnost prurezu:
V(z.Rd)=  277.09 kN
 0.01 < 1 -> Prurez vyhovi
Smykova unosnost prurezu:
V(y.Rd)=   14.90 kN
 0.12 < 1 -> Prurez vyhovi
Momentove unosnosti prurezu
M(c.y.Rd)=   20.90 kNm
M(c.z.Rd)=    4.85 kNm
Interakce:  0.73 < 1 -> Prurez vyhovi

Stabilita dilce:
 0.95 < 1 -> Podminka splnena

Dilec VYHOVI
```

Prurez	- Section
cislo dilce	- Part number
poradnice	- Ordinate
Trida prurezu	- Section class
Spoctena	- Calculated
POSUDEK	- ASSESSMENT
Tlakova unosnost	- Pressure load bearing capacity
Prut na tlak vyhovi	- Rod pressure strength satisfactory
Smykova unosnost prurezu	- Section shear strength
momentove unosnosti prurezu	- Section moment load bearing capacity
Stabilita dilce	- Part stability
Interakce	- Interaction
Podminka splnena	- condition complied with
Prurez vyhovi	- Section satisfactory
Dilec vyhovi	- Part satisfactory

```
Prurez: Q4_ECKS       cislo dilce=  192
A    =       2112.0 mm2poradnice=      0.0 m  N(Ob) = -24.2 kN
Iy   =  1.21680E+7 mm4                       My(Sd)=    3.4 kNm
Iz   =  2.82200E+6 mm4                       My(Ob)=   -6.1 kNm
It   =      33200.0 mm4                      Mz(Sd)=    2.7 kNm
Iw   =  4.4569E+10 mm6                       Mz(Ob)=   -2.9 kNm
Wply=      97829.2 mm3                       Vz(Sd)=    4.1 kN
Wplz=      22688.5 mm3                       Vz(Ob)=   -1.8 kN
Trida prurezu= 3                             Vy(Sd)=    1.8 kN
                                             Vy(Ob)=   -1.5 kN
                                             Mx(Sd)= 1.3E-4 kNm
                                             Mx(Ob)=-3.3E-4 kNm
                                             N max = -24.8 kN
                                             My max=   -6.1 kNm
                                             Mz max=   -2.9 kNm
```

ASSESSMENT - SECTIONS: Q_5

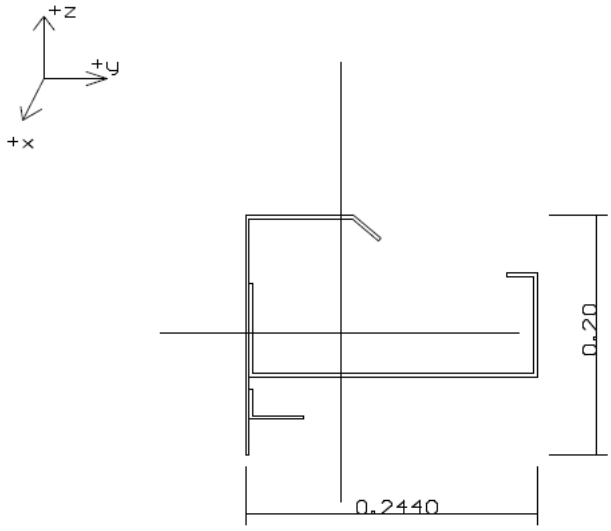



```
POSUDEK
Tlakova unosnost N(b.Rd)=  163.22 kN
 0.00 < 1  -> Prut na tlak vyhovi

Smykova unosnost prurezu:
V(z.Rd)=   56.91 kN
 0.09 < 1 -> Prurez vyhovi
Smykova unosnost prurezu:
V(y.Rd)=   75.67 kN
 0.00 < 1 -> Prurez vyhovi
Momentove unosnosti prurezu
M(c.y.Rd)=    4.28 kNm
M(c.z.Rd)=    0.79 kNm
Interakce:  0.10 < 1 -> Prurez vyhovi

Stabilita dilce:
 0.79 < 1 -> Podminka splnena

Dilec VYHOVI
```

Prurez	- Section
cislo dilce	- Part number
poradnice	- Ordinate
Trida prurezu	- Section class
Spoctena	- Calculated
POSUDEK	- ASSESSMENT
Tlakova unosnost	- Pressure load bearing capacity
Prut na tlak vyhovi	- Rod pressure strength satisfactory
Smykova unosnost prurezu	- Section shear strength
momentove unosnosti prurezu	- Section moment load bearing capacity
Stabilita dilce	- Part stability
Interakce	- Interaction
Podminka splnena	- condition complied with
Prurez vyhovi	- Section satisfactory
Dilec vyhovi	- Part satisfactory

```
Prurez: Q5_FUSS       cislo dilce=   210     N(Sd) =    0.6 kN
A   =        764.0 mm2poradnice=     0.0 m  N(Ob) =   -0.3 kN
Iy  =  8.01000E+5 mm4Ly =      0.0 m        My(Sd)= 2.5E-2 kNm
Iz  =      84900.0 mm4Lz =      0.0 m        Mz(Sd)= 7.6E-2 kNm
It  =       7000.0 mm4Kat. pripoje: neni C  Mz(Ob)=-4.0E-2 kNm
Iw  =  1.17996E+8 mm6                       Vz(Ob)=   -5.2 kN
Wply=      22494.1 mm3                      Vy(Sd)= 6.5E-2 kN
Wplz=       5752.9 mm3                      Vy(Ob)=-3.5E-2 kN
Trida prurezu= 3 (spoctena= 1)              Mx(Ob)=-9.0E-5 kNm
                                            N max =    0.6 kN
                                            My max=   -3.0 kNm
                                            Mz max=-7.6E-2 kNm
```

ASSESSMENT - SECTIONS: Q_6




```
POSUDEK
Tlakova unosnost N(b.Rd)=  516.57 kN
 0.00 < 1  -> Prut na tlak vyhovi

Smykova unosnost prurezu:
V(z.Rd)=  128.41 kN
 0.03 < 1 -> Prurez vyhovi
Smykova unosnost prurezu:
V(y.Rd)=  126.85 kN
 0.02 < 1 -> Prurez vyhovi
Momentove unosnosti prurezu
M(c.y.Rd)=   15.44 kNm
M(c.z.Rd)=   51.84 kNm
Interakce:  0.24 < 1 -> Prurez vyhovi
Stabilita dilce:
 0.27 < 1 -> Podminka splnena
Dilec vyhovi

Dilec VYHOVI
```

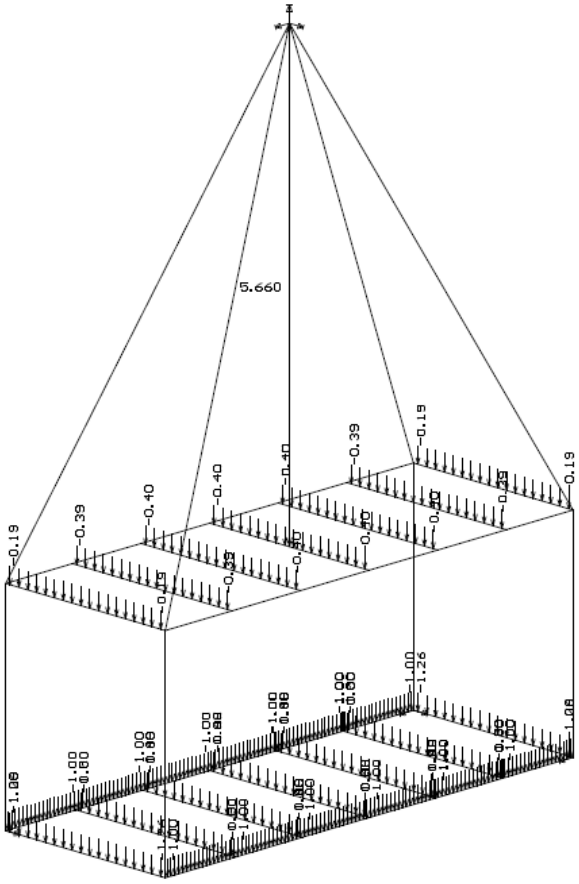
Prurez	- Section
cislo dilce	- Part number
poradnice	- Ordinate
Trida prurezu	- Section class
Spoctena	- Calculated
POSUDEK	- ASSESSMENT
Tlakova unosnost	- Pressure load bearing capacity
Prut na tlak vyhovi	- Rod pressure strength satisfactory
Smykova unosnost prurezu	- Section shear strength
momentove unosnosti prurezu	- Section moment load bearing capacity
Stabilita dilce	- Part stability
Interakce	- Interaction
Podminka splnena	- condition complied with
Prurez vyhovi	- Section satisfactory
Dilec vyhovi	- Part satisfactory

```
Prurez: Q6_OORA       cislo dilce=   224     N(Sd) =    2.0 kN
A   =       2418.0 mm2poradnice=     0.0 m  N(Ob) =   -0.1 kN
Iy  =  7.33000E+6 mm4Ly =      0.0 m        My(Sd)=    3.2 kNm
Iz  =  1.93480E+7 mm4Lz =      0.0 m        My(Ob)=   -3.4 kNm
It  =      32000.0 mm4Kat. pripoje: neni C  Mz(Sd)=    1.5 kNm
Iw  =  1.3664E+11 mm6                       Mz(Ob)=   -2.4 kNm
Wply=      74378.5 mm3                      Vz(Sd)=    2.4 kN
Wplz=  1.17789E+5 mm3                       Vz(Ob)=   -3.3 kN
Trida prurezu= 3                            Vy(Sd)=    1.0 kN
                                            Vy(Ob)=   -2.4 kN
                                            Mx(Sd)= 1.8E-3 kNm
                                            Mx(Ob)=-1.8E-3 kNm
                                            N max =    2.0 kN
                                            My max=   -3.4 kNm
                                            Mz max=   -2.4 kNm
```

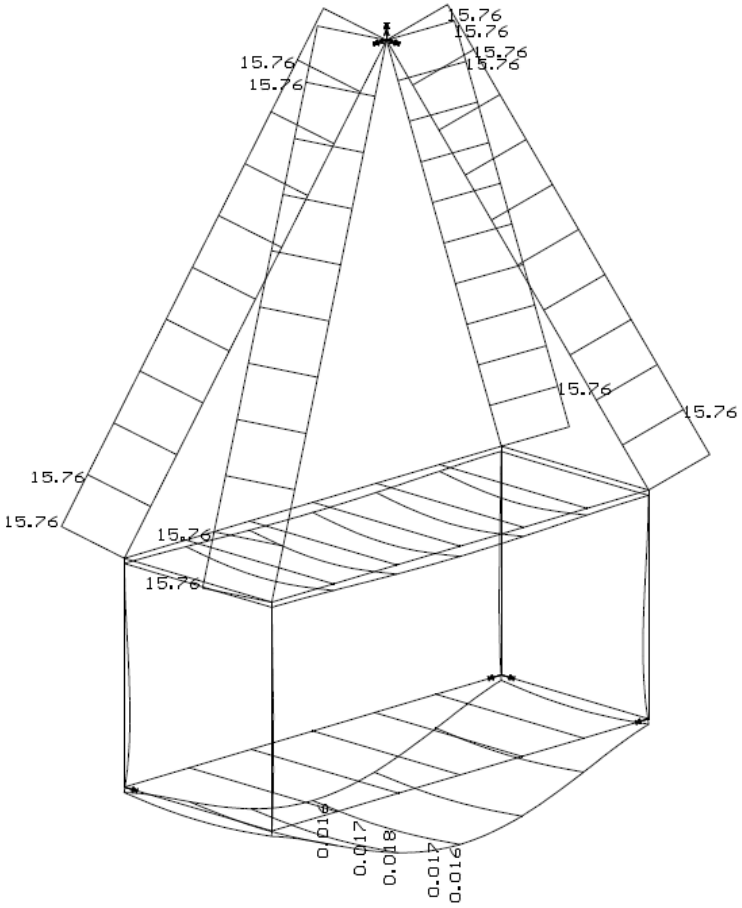
MONTAGEZUSTAND – BELASTUNGSSCHEMA:

L 1 - OWN WEIGHT (VALUES BY INTENDED PROGRAM)

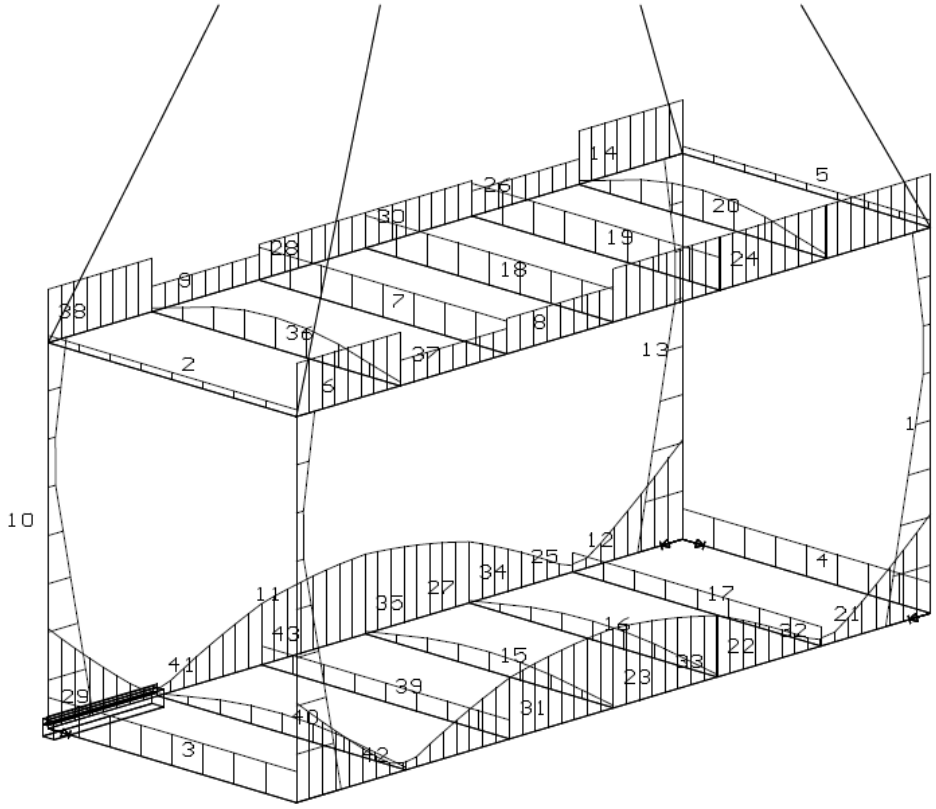
L 2 - ÜBRIGBLEIBENDELASTEN (GEWICHT DER KOMPLETATIONSKONSTRUKTIONEN)



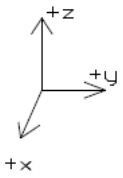
RESULTS – DEFORMATION + NORMAL FORCE:

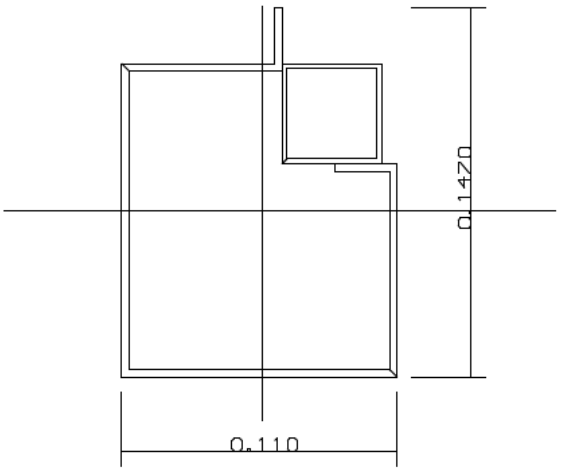


CROSS SECTION LOCATION - CAPACITY UTILIZATION (σ / σ_{RD} ≤ 1):



ASSESSMENT:






```
POSUDEK

Tahova unosnost N(t,Rd)=  333.49kN
 0.01 < 1 -> Prut na tah vyhovi

Smykova unosnost prurezu:
V(z,Rd)=  102.78 kN
 0.07 < 1 -> Prurez vyhovi
Smykova unosnost prurezu:
V(y,Rd)=   91.01 kN
 0.00 < 1 -> Prurez vyhovi
Momentove unosnosti prurezu
M(c,y,Rd)=    9.48 kNm
M(c,z,Rd)=   10.17 kNm
Interakce:  0.69 < 1 -> Prurez vyhovi

Dilec VYHOVI
```

Prurez	- Section
cislo dilce	- Part number
poradnice	- Ordinate
Trida prurezu	- Section class
Spoctena	- Calculated
POSUDEK	- ASSESSMENT
Tlakova unosnost	- Pressure load bearing capacity
Prut na tlak vyhovi	- Rod pressure strength satisfactory
Smykova unosnost prurezu	- Section shear strength
momentove unosnosti prurezu	- Section moment load bearing capacity
Stabilita dilce	- Part stability
Interakce	- Interaction
Podminka splnena	- condition complied with
Prurez vyhovi	- Section satisfactory
Dilec vyhovi	- Part satisfactory

```
Prurez: O3_UL8QRA     cislo dilce=    29      N(Sd) =    3.6 kN
A    =       1561.0 mm2poradnice=      0.0 m  My(Sd)=    6.6 kNm
Iy   =  3.57500E+6 mm4Kat. pripoje: neni C   Mz(Sd)= 3.4E-2 kNm
Iz   =  2.66800E+6 mm4                       Vz(Sd)=   -7.4 kN
It   =     23800.0 mm4                       Vy(Sd)= 3.1E-2 kN
Iw   =  1.5501E+10 mm6                       Mx(Sd)= 4.2E-4 kNm
Wply=     44387.9 mm3                       N max =    3.6 kN
Wplz=     49462.4 mm3                       My max=    6.6 kNm
Trida prurezu= 3                            Mz max= 3.4E-2 kNm
```